

Report on Groundwater Modelling of Potential Development Impacts

Proposed Manufactured Housing Estate

40-80 and 82 Chapmans Road, Tuncurry

Prepared for Allam MHE Developments No. 2 Pty Ltd

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The undersigned, on behalf of Douglas Partners Pty Ltd, confirm that this document and all attached drawings, logs and test results have been checked and reviewed for errors, omissions and inaccuracies.

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Douglas Partners acknowledges Australia's First Peoples as the Traditional Owners of the Land and Sea on which we operate. We pay our respects to Elders past and present and to all Aboriginal and Torres Strait Islander peoples across the many communities in which we live, visit and work. We recognise and respect their ongoing cultural and spiritual connection to Country.



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Report on Groundwater Modelling of Potential Development Impacts Proposed Manufactured Housing Estate 40-80 and 82 Chapmans Road, Tuncurry

1. Introduction

This report presents the results of groundwater modelling of potential development impacts undertaken for the proposed manufactured housing estate at 40-80 and 82 Chapmans Road, Tuncurry. The investigation was commissioned by Allam MHE Developments No. 2 Pty Ltd (Allam) on 26 November 2024and was undertaken in line with Douglas Partners Pty Ltd (Douglas) proposal 2195396.00.P.005.Rev0 dated 26 November 2024. The site is shown on Drawing 1, Appendix A.

It is understood that the proposed development comprises a manufactured housing estate which will be constructed on imported fill. There has been no specific request for information (RFI) from Mid Coast Council (MCC) regarding groundwater for the proposed development at this stage, however, previous submissions between Allam and MCC have resulted in RFI from MCC. The purpose of this report is to provide information to MCC in anticipation of MCC comments/RFI.

This report should be read in conjunction with all appendices including the notes provided in Appendix B.

2. Scope of Work

The scope of work comprised following:

- Brief review of available existing data comprising geological maps, topographic data, registered groundwater bores and salinity mapping;
- Summary of subsurface conditions from previous investigations by Douglas;
- Preparation of a conceptual hydrogeological model (CHM);
- Groundwater modelling of the site under two scenarios (pre-development and post development) using MODFLOW; and
- Preparation of this report including results of groundwater modelling, estimated groundwater impacts and recommendations for future work.

For the purposes of this report the following documents were provided:

- ADW Johnson (ADWJ) Detail and Contour Survey 190835-DET-001-A, revision A;
- Concept Engineer Plans ADWJ 190835-CENG- revision A dated 09/12/2024;
- Master Plan ADWJ 190835-MP- revision K dated 09/12/2024;
- Catchment Plans ADWJ 190835-WCMP revision A dated 29/11/2024;
- Regional Geotechnical Solutions Pty Ltd (RGS) Detailed Site Investigation (RGS, 2022); and



• Regional Geotechnical Solutions Pty Ltd (RGS) Addendum to Detailed Site Investigation – Contamination Assessment (RGS, 2023).

3. **Proposed Development**

Reference to provided Master Plan (ADWJ – 190835-MP- revision K dated 09/12/2024) design surface provided by ADWJ and recent discussions with ADWJ, it is understood that the proposed development comprises the following (see Figure 1):

- Construction of engineered fill platform up to about 3.5 m above current surface (design RL 2 to RL 5 m AHD) across majority of the site, sand or other MCC approved fill material;
- Construction of 283 manufactured homes, community centre and internal roads on the fill platform;
- Construction of access road to off Chapmans Road; and
- Stormwater infrastructure (see below for detail).

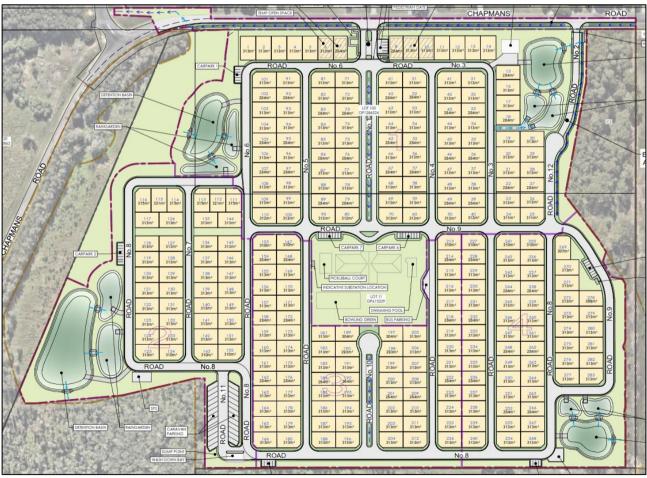


Figure 1: Proposed development (Master Plan – ADWJ – 190835-MP- revision K dated 09/12/2024)



Reference to Stormwater Plans (ADWJ drawing 190835-CENG-401 to 414 revision A) on site

stormwater is managed in six catchments ('Catchment 1' to 'Catchment 6'). Stormwater from Catchment 1 to Catchment 4 is managed on site with stormwater from each catchment managed with two raingardens and one detention basin whereas Catchment 5 and Catchment 6 drain to existing stormwater infrastructure along Chapmans Road. In a storm event which overflows the detention basins

- Catchment 1 drains to Chapmans Road;
- Catchment 2 drains to a C2 zoned piece of land in the south-west corner of the site;
- Catchment 3 drains to a C2 zoned piece of land in the north-east corner of the site; and
- Catchment 4 drains to a C2 zoned piece of land in the south-east corner of the site.



Figure 2: Proposed stormwater catchments ADWJ drawing 190835-CENG-401 revision A dated 09/12/2024

Reference to Stormwater Plans and recent discussions with ADWJ the proposed stormwater management for each catchment (except Catchment 5 and 6) is as follows:

- Stormwater collected from the catchment via pit and pipe.
- Stormwater enters a 'splitter pit' and is split between two raingardens ('A' and 'B'). The raingardens are un-lined and infiltrate run-off through filter media into the underlying subsurface. The raingardens have been designed handle less than 3 month annual return interval (ARI) storm events.



- In the event of a >3 month ARI storm event, the stormwater is designed to overflow from the raingardens via a 3 m wide spillway into a single detention basin which is un-lined and infiltrates run-off through filter media (same as raingardens).
- In an emergency, stormwater can overflow the detention basin via a 5m wide spillway and level spreader.

Therefore, under average rainfall conditions, it would be expected that the rain gardens accept all site stormwater and the detention basins are not utilised.

Pertinent details obtained from the stormwater plans are summarised in Table 1 below.

Table 1: Raingarden and detention basin details (ADWJ drawing 190835-S2-WCMP-001 to 009 Rev A dated 29/11/2024)

	Catabra ant	Raing	arden	Detention basin		
Catchment	Catchment area (Ha)	Base RL	Total base area (m²)*	Base RL	Approx base area (m²)**	
1	4.4	2.4	697	2.0	769	
2	6.3	2.3	1411	1.9	2286	
3	2.0	3.1	490	2.7	640	
4	2.63	3.1	665	2.7	740	

Notes to table:

* Sum of Raingarden A and Raingarden B

** Base area scaled off Stormwater Plans and therefore approximate only.

RL Reduced level (m AHD)

4. Site Description

Site identification information is presented in Table 2, with site shown in aerial (Figure 3) and drone photo (Figure 4) below.

Table 2: Site Identification

Item	Details
Allotment Identification	Lot 100 DP 1286524 and Lot 11 DP 615229
Street Address	40-80 Chapmans Road and 82 Chapmans Road
Locality	Tuncurry, NSW
Site Area (approximately)	22.5 ha
Local Government Area	Mid Coast Council (MCC)
Current Zoning	R2 – Low density residential E2 – Environmental conservation
Current Land use	Vacant





Figure 3: The 'site', approximate site boundary shown in red (base map from Sixmaps)



Figure 4: The 'site' shown in red (photo provided by ADW Johnson dated 19 November 2024)



At the time of most recent field work (5 November 2024) the lot was undeveloped with a fill platform approximately 1 m thick in the central portion of the site (see Figure 4 above). 'Enddump' stockpiles of soils and stockpiles of materials (pallets, concrete pits, brick, sheet metal, wood) are in the central portion of the site, on top of the fill platform.

Additionally, there was a stockpile of sand in the eastern portion of the site (see Figure 4 above) which was observed by Douglas to be imported during December 2023 field work. Site personnel at the time indicated this sand material was sourced from dredging of Coolongolook River nearby Forster Tuncurry Bridge.

The eastern and western portions of the site did not appear to be filled and were undeveloped with stockpiles of mulch, probably from recent removal and subsequent mulching of mature vegetation such as trees. Vegetation across the site comprised mainly 'slashed' grass and some denser bushes typically on soil stockpiles.

5. **Published Data**

5.1 **Topography**

Detailed survey of the site was only available for the north eastern area of the site. Reference to detailed survey by ADW (project 190835-DET-001-A, copy provided in Appendix A) indicates the site levels as of July 2022 are as follows:

- Western and eastern portion of the site in the order of RL 0.5 (m AHD) to RL 1.5; and
- Central portion of the site in the order of RL 1.5 to RL 4.5.

Reference to NSW Spatial Services 1 m Digital Elevation Model (DEM) for Bulahdelah and Forster dated 2012 indicates the following:

- The site has an elevated area in the north east (see above detailed survey), generally the remainder of the site is at around RL 2.5 in the eastern area of the site sloping down to the west to a minimum of around RL 0.5; and
- Regional topography to be in the order of RL 0 at Wallamba River to the west of the site, grading to RL 7 to the east of the site, with an overall slope dipping to the west/south west.

5.2 Geology

Reference to NSW Seamless Geology map indicates the following (see Figure 5):

- Quaternary aged coastal deposits (QH_br in Figure 5), which typically comprises sand, shell and gravel eastern area of the site;
- Quaternary aged estuarine tidal-delta flat (QH_et in Figure 5), which typically comprises sand, silt, clay shell and gravel central area of the site; and
- Quaternary aged clastic sediments (QH_af), which typically comprise silt, sand and clay on some western parts of the site.



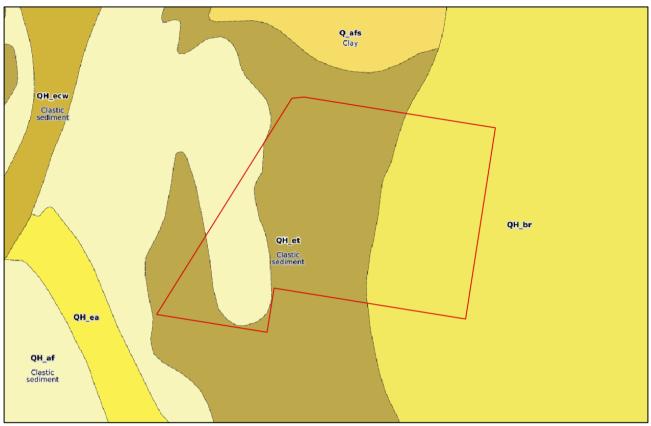


Figure 5: Geology mapping, site shown in red

5.3 **Registered groundwater bores**

An on-line records search of groundwater wells registered with the NSW Office of Water indicated the absence of registered wells on the site (see Figure 6).





Figure 6: NSW registered groundwater bores, site boundary shown in red outline.

Numerous registered wells were within 500 m of the site, details are summarised below in Table 3.



Bore ID	Easting, northing	Bore depth (m bgl)	Depth to GW (m bgl)	Subsurface logged (m bgl)	Purpose	Distance from site (m)
GW200229	451773, 6441839	9.0	2.7	Sand (0-9.0)	Recreational	400 m south-east
GW080297	451877, 6442028	9.0	2.7	Sand (0-9.0)	Not available	450 m east
GW201112	451123, 6442650	7.0	2.0	Not available	Irrigation	480 m north
GW047063	451593, 6442629	8.0	-	Soil humus (0-0.3) Sandstone (0.3-8.0)	Mining	550 m north
GW047062	451619, 6442691	8.0	-	Soil humus (0-0.3) Sandstone (0.3-8.0)	Mining	600 m north
GW078865	451443, 6441089	3.0	1.2	Gravel/sand (0-3.0)	Monitoring	650 m south
GW202224	451984, 6441465	7.0	-	Topsoil/sand (0-7.0)	Domestic	700 m south east
GW078867	452095, 6441769	7.5	2.73	Sand (0-7.5)	Monitoring	750 m east
GW78858	451915, 6440968	22.0	-	Sand (0-21.0) Clay (21.0- 22.0)	Monitoring	950 m south east

Table 3: Registered groundwater monitoring wells (within 1km of the site)

Notes to table:

m bgl metres below ground level GW Groundwater

The registered bores within the vicinity of the site generally indicate a sandy subsurface profile with groundwater observed during drilling at depths between 1.2 m and 2.73 m. It should be noted that groundwater levels are affected by factors such as climatic conditions and soil permeability and therefore vary with time.

Additionally, clay was recorded on the drilling log for GW78858 at a depth of 21 m.

5.4 Surface water

The surface water features in the vicinity of the site as follows:

- Table drain along Chapmans Road, directly north and west of the site;
- Unnamed channel, approximately 25 m south of the site;
- Lake at the racecourse, approximately 150 m north of the site; and



• Wallamba River, approximately 475 m west of the site.

The local surface flow direction is likely to be to the west (toward Wallamba River).

5.5 Salinity

With reference to NSW Department of Planning and Environment eSPADE mapping indicates a data node approximately 25 m north-west of the site has "salting evident". This suggests potentially high salinity conditions in soils within the vicinity of the site.

5.6 Acid sulfate soils

Reference to NSW Acid Sulfate Soil Risk map indicates that there is a high probability of acid sulfate soil occurrence within 1 m of the ground level at the site (see Figure 7).



Figure 7: High probability of ASS (in red), low probability of ASS (in orange), site boundary shown in black outline

5.7 Groundwater dependant ecosystems (GDEs)

A review of the Bureau of Meteorology (BOM) Groundwater Dependant Ecosystems Atlas indicates the following:

- On site:
 - The south western, north eastern and south eastern areas of the site are mapped as low to high potential for terrestrial GDE (swamp paperbark, swamp mahogany, swamp oak, saw sedge, baumea juncea grasses, banksia) (see red and orange areas on Figure 8);



- Off site:
 - Various areas in the vicinity of the site are mapped as low to high potential for terrestrial GDE (swamp paperbark, swamp mahogany, swamp oak, saw sedge, baumea juncea grasses, banksia) (see red and orange areas on Figure 8);
 - Wallamba River (approximately 300 m west of the site) as high potential for aquatic GDE (see blue area on Figure 8).



Figure 8: High potential for terrestrial GDE (red), medium potential for terrestrial GDE (orange), high potential for aquatic GDE (blue), site boundary shown in red outline.

6. **Previous Investigations**

6.1 Detailed site investigation – Regional Geotechnical Solutions (RGS, 2022; RGS, 2023)

Regional Geotechnical Solutions (RGS) has undertaken a detailed site investigation for contamination at the site in 2022 (RGS, 2022). Following council comments an addendum to the detailed site investigation was also undertaken (RGS, 2023). Scope of work for this previous investigation by RGS included site walkover, excavation of 55 test pits, installation of four wells, soil and water sampling and analysis of collected samples for testing.

During the site walkover by RGS on 4 October 2022 the site was described as follows:

- The eastern and western portions of the site are cleared of mature trees with mulching in progress;
- The central portion of the site has been extensively filled above natural grade;



- An asphalt access road, from Chapmans Road to the central portion of the site; and
- Various stockpiles of materials such as aggregate, road base, gravel, vegetation and anthropogenic materials scattered across the site.

Subsurface conditions encountered in test pits generally comprised:

- Fill:
 - In above ground stockpiles, generally in the eastern portion of the site, generally comprising of sandy gravelly CLAY / sandy GRAVEL / clayey gravelly SAND / sandy GRAVEL;
 - o In the central portion of the site, generally comprising SAND with some shell; underlain by
- Sand / clayey sand (slightly indurated in areas): grey / pale grey to termination depth with maximum investigation to 2.6 m below ground level.

Groundwater monitoring wells (MW1 to MW4) were installed on 7 September 2022 by RGS. The monitoring wells were installed using a 6T Excavator with 100 mm diameter auger to depths of 2.0 m. Locations of MW1 to MW4 are included on Drawing 1, Appendix A and borehole logs are in Appendix C.

Subsurface conditions encountered in MWI to MW4 generally comprised:

- Fill / topsoil / clayey sand: dark grey / black, some roots to depths of between 0.2 m to 0.7 m; underlain by
- Sand / clayey sand (slightly indurated in areas): pale grey / grey / pale brown / dark brown to the termination depth of 2.0 m.

Groundwater was observed at depths between 0.5 m and 1.0 m during drilling.

Groundwater monitoring well construction details were not provided on borehole logs by RGS. MW1, MW3 and MW4 were inspected by Douglas in May 2023, at the time of inspection MW2 was destroyed. A summary of the observed monitoring well construction is provided in Table 4.

Well	Depth (m)	Depth to bottom of screen (m)		
MW1	2.0	1.5	2.0	
MW2	Destroyed prior to May 2023			
MW3	2.0	1.5	2.0	
MW4	2.0	1.5	2.0	

Table 4: Groundwater monitoring well construction (MW1, MW3 and MW4)

Notes to table:

Screen lengths were measured with a steel tape and are therefore approximate only.

It should be noted that groundwater levels are affected by factors such as climatic conditions and soil permeability and will therefore vary with time.



6.2 Groundwater study and preliminary groundwater investigation – Douglas (Douglas, 2023; Douglas, 2024a)

Douglas has previously completed a Groundwater Study for 40-80 Chapmans Road, Tuncurry (Douglas, 2024a) and a Preliminary Groundwater Investigation at 82 Chapmans Road, Tuncurry (Douglas, 2023). Pertinent results are summarised below.

The general subsurface conditions at Bore 101 to Bore 104 comprised the following:

- Sandy silt/silty sand (topsoil): Generally comprising dark brown, sandy silt/silty sand with rootlets to depths of between 0.1 m and 0.2 m below ground level (bgl). Encountered in all bores except Bore 102;
- Silty sand/sand: Dark brown/brown/pale brown/pale grey, silty sand / sand, with varying proportions of silt to the termination depth of 2.75 m bgl (limit of investigation).

Locations of MW1, MW3, MW4 and 101 to 105 is shown in Drawing 1, Appendix A and borehole logs are in Appendix C.

In-situ hydraulic conductivity testing was completed at MW1, MW3, MW4 and 101 to 104 during field work. Copies of the analysis sheets are in Appendix C with the results are summarised below.

	Matarial	Well construction				Estimated horizontal hydraulic conductivity (Hvorslev, 1951)	
Well	Material screened	Casing radius (m)	Gravel radius (m)	Effective screen interval (m)	Number of tests	m/second	m/day
MWI	Sand	0.025*	0.075*	1.5*	4	0.1 to 1.3 x 10 ⁻⁴	1 to 11
MW3	Clayey sand	0.025*	0.075*	1.5*	4	2.0 to 7.7 x 10 ⁻⁵	2 to 7
MW4	Clayey sand	0.025*	0.075*	1.5*	7	0.3 to 1.6 x 10 ⁻⁴	5 to 14
101	Sand	0.025	0.075	1.56	4	7.3 to 8.8 x 10 ⁻⁵	6 to 8
102	Sand	0.025	0.075	1.65	3	1.6 to 3.8 x 10 ⁻⁵	1 to 3
103	Sand	0.025	0.075	1.75	4	6.5 to 7.4 x 10 ⁻⁵	6 to 7
104	Sand	0.025	0.075	1.75	4	0.9 to 1.1 x 10 ⁻⁴	8 to 10

Table 5: Summary of in-situ hydraulic conductivity tests (Douglas, 2023; Douglas, 2024a)

Notes to table:

* Well construction detail not available, value assumed

In-situ screening of surface waters for field parameters, measured using a hand-held calibrated meter on 1 May 2023 (locations W1 to W13) and 28 June 2023 (locations W101 and W102). The locations were screened to give a broader understanding of water quality in nearby surface water bodies. Locations are shown in Figure 9 and results are summarised in Table 6.





Figure 9: Approximate location of surface water screening (W1 to W13, W101 and W102)



Table 6: Summary of Field screening of surface waters (Douglas, 2023; Douglas, 2024a) - W1 to W13 (1/5/2023) and W101 to W102 (28/6/2023)

	Location		Easting	Northing	RL (AHD)	Temperature	рН	EC (mS/cm)	ORP (mV)	DO (mg/L)	Turbidity (NTU)
On site	W1	Low lying area, south eastern portion of the site	451321	6441984	1.4	24.5	7.0	0.27	81	5.8	1
On site	W2	Low lying area, north eastern portion of the site	451367	6442063	1.5	21.3	5.6	0.29	154	4.9	28
On site	W3	Low lying area, northern boundary of the site	451230	6442138	1.1	21.2	6.2	0.60	-3	1.1	>1000
On site	W4	Low lying area, northern boundary of the site	451180	6442152	1.1	22.5	6.3	0.51	-25	2.3	157
On site	W101	Surface water body, western portion of the site	450929	6441937	-	11.7	8.0	0.408	67	11.6	110
On site	W102	Surface water body, eastern portion of the site	451370	6441908	-	12.3	6.3	0.111	155	5.9	410
Off site	W5	Surface water body, Tuncurry Lakes Resort	450423	6441736	0.1	22.6	6.8	23.00	83	5.4	2
Off site	W6	Wallamba River	450292	6441654	0.1	22.2	7.0	21.20	54	6.4	5
Off site	W7	Table drain, north side of Chapmans Road	450625	6441744	0.5	19.4	7.5	0.97	-19	6.7	3



Table 6: Summary of Field screening of surface waters	(Douglas, 2023; Douglas, 2024a) - W1 to W13 (1/5/2023) and W101 to W102
(28/6/2023)	

	Location		Easting	Northing	RL (AHD)	Temperature	рН	EC (mS/cm)	ORP (mV)	DO (mg/L)	Turbidity (NTU)
Off site	W8	Table drain, south side of Chapmans Road	450700	6441820	0.6	20.5	6.8	0.07	3	6.5	42
Off site	W9	Table drain, south side of Chapmans Road	450816	6441987	0.5	20.1	7.4	0.42	21	6.9	43
Off site	W10	Surface water body, racecourse	451320	6442876	0.5	21.4	8.0	0.35	42	10.3	7
Off site	WII	Concrete lined drain adjacent to Viola Circuit	451670	6441747	1.8	19.7	7.7	0.26	56	6.0	54
Off site	W12	Wallamba River	451278	6439501	0.3	21.4	7.4	41.00	115	6.6	6
Off site	W13	Coolongolook River	452842	6439352	0.3	21.5	7.8	49.40	79	6.5	2

Notes to table:

Co-ordinates of tested locations were measured using a hand-held GPS and therefore coordinates are approximate only.

EC – electrical conductivity

NTU – Nephelometric Turbidity Units

DO – dissolved oxygen

ORP – oxidation reduction potential



6.3 Infiltration testing – Douglas (Douglas, 2024b)

Douglas has previously conducted infiltration testing at the site at 12 locations (201 to 212). The location of 201 to 212 is shown in Drawing 1, Appendix A. Infiltration testing included excavation of a test pad to a depth below topsoil, double ring infiltrometer testing in the stripped surface, and drilling of a hand auger (to observe groundwater). The testing included testing within existing site filling, as well as within natural soils.

Locations of 201 to 212 is shown in Drawing 1, Appendix A and borehole logs and double ring infiltrometer sheets are in Appendix C with the results are summarised below in Table 7 and groundwater observations during testing in Table 7.

Location	Soil tested	Depth tested	Estimated infiltration rate		Estimated vertical saturated hydraulic conductivity*		
		m	m/s	mm/hour	m/s		
201	Fill/sand	0.5	3.6 x 10 ⁻⁴	1290	3.3 x 10 ⁻⁴		
202	Sand with silt	0.2	2.2 x 10⁻⁵	79	1.9 x 10 ⁻⁵		
203	Sand	0.2	1.7 x 10 ⁻⁴	597	1.5 x 10 ⁻⁴		
204	Sand	0.2	2.8 x 10 ⁻⁵	99	1.4 x 10⁻⁵		
205	Sand	0.2	6.6 x 10 ⁻⁶	23	2.2 x 10 ⁻⁶		
206	Sand	0.3	5.2 x 10 ⁻⁵	186	4.0 x 10 ⁻⁵		
207	Fill/sand	0.1	5.7 x 10 ⁻⁴	2058	5.2 x 10 ⁻⁴		
208	Fill/sand	0.2	8.0 x 10 ⁻⁴	2872	6.4 x 10 ⁻⁴		
209	Fill/sand	0.0	9.5 x 10 ⁻⁴	3412	8.6 x 10 ⁻⁴		
210	Fill/sand	0.2	1.2 x 10 ⁻⁴	449	1.2 x 10 ⁻⁴		
211	Clayey sand	0.3	Infiltration not observed over 45 minutes				
212	Clayey sand	0.2	Infiltration not observed over 20 minutes				

Table 7: Summary of double ring infiltrometer testing

Notes to table:

Estimated using the method presented in a paper in the Journal of Geotechnical Engineering (Daniel, D.E., 1989).



Bore	Surface level (AHD)	Depth to groundwater (m)	Groundwater level (AHD)
201	2.1	NE to 1.0	-
202	1.5	0.90	0.55
203	1.7	0.80	0.89
204	1.1	0.30	0.76
205	1.0	0.45	0.58
206	1.3	0.60	0.67
207	2.1	NE to 0.9	-
208	2.9	NE to 0.4	-
209	2.7	NE to 0.3	-
210	2.3	NE to 1.2	-
211	0.7	0.35	0.36
212	0.8	0.35	0.40

Table 8: Summary of groundwater observations (30/11/2023 and 1/12/2023)

Notes to table:

NE Not encountered

6.4 Groundwater monitoring – Douglas (Douglas, 2024c)

Douglas has conducted groundwater level monitoring at the site with dataloggers at eight locations across the site. Groundwater level is monitored using a datalogger which is installed in each monitoring well and measures water pressure at 20-minute increments.

The purpose of the monitoring is for the assessment and comment on short-term and long-term trends in groundwater levels (groundwater fluctuations). Monitoring conducted to date is summarised in Table 9.



Well	Monitoring commenced	Monitoring concluded	Total months of data capture
MW1	May 2023	November 2024	19*
MW3	May 2023	Ongoing	19
MW4	May 2023	Ongoing	19
101	August 2023	Ongoing	16
102	August 2023	Ongoing	16
103	August 2023	Ongoing	16
104	August 2023	Ongoing	16
105	August 2024	Ongoing	3

Table 9: Summary of groundwater monitoring conducted to date

Notes to table

MWI datalogger malfunctioned between November 2023 to August 2024 therefore data is approximate only.

Summary of groundwater depths and levels measured to date is provided in Table 10.

Well	Well depth	Range of ground (m)	-	Range of groundwater level (RL - AHD)		
	(m)	Min	Max	Min	Max	
MWI	2.0	-0.2*	0.9	0.6	1.7*	
MW3	2.0	0.0	0.8	0.3	1.1	
MW4	2.0	0.5	1.3	0.2	1.1	
101	2.75	0.0	1.1	0.7	1.8	
102	2.75	0.1	1.3	0.5	1.6	
103	2.75	-0.2	0.6	0.2	0.9	
104	2.75	-0.2	0.5	0.1	0.8	
105	1.93	0.5	0.8	1.0	1.3	

Table 10: Summary of groundwater depth and level (MW1, MW3, MW4 and 101 to 105)

Notes to table

RL Reduced level (m AHD)

m bgl metres below ground level

Datalogger malfunction, therefore is considered approximate only

Negative depths indicate ponding above the surface

For rainfall up to 5 February 2024 the data is plotted against composited rainfall data from five Bureau of Meteorology (BOM) weather stations at (Wootton, Bungwahl, Old Bar, Forster and Taree Airport). On 6 February 2024 a site rainfall gauge was installed by ADW Johnson and therefore from 6 February 2024 onwards the data is plotted against site rainfall gauge. Datalogger data collected to date, plotted against rainfall is included in Appendix C.



7. Groundwater Flow

Estimated groundwater contours representing conditions on 5 November 2024 are presented on Figure 10 and are based on groundwater levels measured at MW1, MW3, MW4, 101 to 105 on 5 November 2024.

The relative differences in groundwater levels between monitored wells has been generally maintained throughout groundwater monitoring to date (see Figure 1 and Figure 2, Appendix C).

Estimated groundwater contours from manual gauging on 5 November 2024 are shown in Figure 10. Groundwater contours suggest groundwater to be flowing to the west on the date dipped.



Figure 10: Estimated groundwater contours 5 November 2024 in blue, inferred groundwater flow direction shown as red arrows, site boundary shown in red outline.

8. **Conceptual Hydrogeological Model**

8.1 Pre-development

Groundwater strata beneath the site comprise unconfined aquifers within alluvial/aeolian sands of relatively high permeability (see Section 6.2). This unconfined aquifer is understood to be primarily sandy with a high transmissivity for groundwater flow, noting that permeability of soils may decrease with clastic sediments to the west of the site (closer to Wallamba River). Lower permeability layers or lenses, comprising silt or indurated sand may also be present, but these are not expected to be confining layers.



Based on previous site investigations in Tuncurry by Douglas, the base of the unconfined aquifer is at least 10 m deep, with registered bore GW78858 about 950 m south east of the site suggesting clay at a depth of 21 m (see Section 5.3).

Groundwater flow is typically following surface topology, which is generally to the west (see Section 7), however flow directions may potentially be dependent on seasonal conditions.

The groundwater monitoring for the site to date indicates the groundwater at the site within less than a metre of the surface during dryer periods and reaching the ground surface during wetter periods such as occurred in April and May 2024, reaching the surface. Shallower groundwater is present in the western area of the site (Well 103 and 104), where groundwater levels were observed to be at or above the ground surface on occasion, particularly often between May 2024 to November 2024. Additionally, lower infiltration rates were observed during infiltration testing in the western area of the site. These lower rates may be attributable to the shallow depth to water table in that area of the site.

Groundwater levels are responsive to rainfall events and the groundwater levels fall relatively quickly after these events, generally returning to previous levels within about a week in the absence of additional rainfall Based on analyses of a selection of isolated rainfall events (10 and 19 February 2024, 4 and 30 April 2024 and 23 October 2024) which coincide with site rainfall gauge readings of between 31 mm to 187 mm per day, the ratio of the response of groundwater level compared to rainfall amount for each event ranged from about 1 to 10 across monitored wells. Assuming 30% recharge, this is equivalent to a specific yield of between about 0.04 and 0.35.

Groundwater recharge will generally be controlled by rainfall. Surface water bodies within the site may be sources of recharge and may locally mound groundwater. The surface water body at the racecourse to the north of the site may be an additional source of recharge and discharge and may impact groundwater flow directions.

During and after relatively wet rainfall conditions groundwater levels can be expected to come close to or above the surface and groundwater flows may be locally affected by surface drainage features (if any) or surface topography, and may follow the surface water flow directions as shown by the blue arrows on Drawing 1, Appendix A. Due the high transmissivity of the aquifer, as well as likely high evapotranspiration levels due to shallow groundwater, levels can be expected to fall relatively quickly in periods of drier rainfall conditions.

When groundwater levels fall below the surface then groundwater flow directions are likely to become more regional and controlled by the larger permanent surface water features, with flow approximately to the west as shown by the red arrows on Drawing 1, Appendix A.

A residual rainfall mass balance (RRMB) has also been undertaken on the long-term rainfall records from Taree Airport (AWS), which have been collected by BOM continuously since 1997 to November 2024. A RRMB provides a cumulative plot of above or below average rainfall over the length of the rainfall records. A slope upwards on the plot indicates above average rainfall whereas a slope downward indicates below average rainfall (over the length of rainfall records). The RRMB calculated from Taree Airport (AWS) rainfall records is shown in Figure 11.



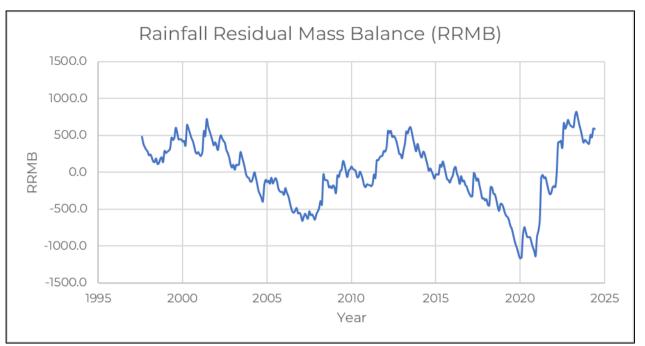


Figure 11: Residual rainfall mass balance (RRMB) from Taree Airport (AWS).

The RRMB plot indicates generally increasing overall trend in rainfall since about 2021, with below average rain for much of the monitoring period since mid-2023. September/October 2024 have seen upwards trending above average rainfall.

Groundwater levels in many aquifers often have similar trends to the RRMB trend line, however in this case the aquifer is highly constrained by boundary conditions, especially the surface, and therefore water levels are more related to shorter terms rainfall events. Lower bound water levels are constrained by the relatively close river with groundwater levels after extended periods unlikely to drop below average tide levels.

8.2 **Post-development**

The following components of the proposed development have the potential to impact groundwater:

- The construction of impervious surfaces such as building roofs, footpaths and roads will reduce both recharge and evapotranspiration on much of the site and concentrate recharge into raingardens/basins, resulting in groundwater mounding;
- Vegetation removal (trees, shrubs, grass etc) may reduce evapotranspiration and infiltration rates whilst maintaining/increasing recharge, resulting in groundwater mounding;
- Filling above current site levels provides the opportunity for elevated groundwater within fill;
- Raingardens/detention basins that infiltrate run-off into the unconfined aquifer may impact groundwater flows and quality;
- If there are no controls, potential contamination to groundwater/surface water from site filling during and post development.



9. Numerical Modelling

9.1 Overview

A steady state groundwater model was considered appropriate to approximate long term average groundwater levels at the site. The purpose of developing the steady state model was to approximately replicate the existing groundwater flow regime on the site and then use this model to estimate the potential for changed groundwater conditions as a result of the proposed development under steady state conditions.

Based on the conceptual groundwater model, steady-state groundwater flow models were developed using the graphical user interface Visual MODFLOW Flex using MODFLOW 2005 numerical engine.

9.2 Model cases

To estimate the degree of groundwater mounding prior to development and after development using a steady state model, two cases were modelled. The cases are as follows:

- Pre-development the current site and surrounds; and
- Post-development after filling of the site and construction of manufactured housing estate.

Additionally, a sensitivity analysis was undertaken to understand the predictive modelling sensitivity to variability in seasonal conditions. Sensitivity analysis was undertaken for the following scenarios:

- Wet average conditions assuming 30% increase in rainfall compared to the base case; and
- Dry average conditions assuming 30% decrease in rainfall compared to the base case.

9.3 Model layers

The groundwater system was represented by a single layer of sand which represents the unconfined aquifer. Based on previous site investigations in Tuncurry by Douglas, the base of the unconfined layer at the site has been assumed to be at RL - 20. It is noted that the bottom of unconfined aquifer may be shallower or deeper at the site however for this report, the depth of the natural sands are not considered to be a sensitive component of the modelling.

The existing surface level of the model was based on NSW Spatial Services 1 m Digital Elevation Model (DEM) for Bulahdelah and Forster dated 2012. Contours of the surface elevation are shown below in Figure 21.





Figure 12: Pre development ground surface, 1 m contours (m AHD) imported into MODFLOW, site boundary shown in red outline

9.4 Model extents and boundary conditions

The extent of the model was set to boundaries which could be reasonably defined as follows:

- The western extent of the model was approximately 500 m from site at Wallamba River and set as a constant head boundary condition of RL 0.5;
- The eastern extent of the model was approximately 2000 m from the site at Pacific Ocean and set as a constant head boundary condition of RL 0.5;
- The remaining extents were a minimum of 1500 m from the site. These boundaries are significantly distant from the site to reduce impacts on the model at the subject site.

The overall size of the model was 5 km from the east to west and 4 km from the north to south, discretised using a non-uniform grid (focused on the site becoming coarser with distance from the site) of 602 columns and 712 rows rotated 80 degrees to approximately align parallel to



Chapmans Road. The grid size was a maximum of 200 m by 200 m (off site) grading to a minimum of 1 m by 1 m (on-site). The size of the grid was selected to allow reasonable representation of the site, site specific features and proposed features including proposed raingardens.

9.5 Model parameters

Hydraulic parameters required for the model included hydraulic conductivity. Initial estimates were obtained from the particle size distribution and slug tests. The permeability of the sand was found to be in the range of 1.6 x 10^{-5} m/sec to 1.6 x 10^{-4} m/sec (geometric mean of 6.3 x 10^{-5}). Parameters used in the development of the model were:

- Horizontal Hydraulic conductivity (KH) = 6.3 x 10⁻⁵ m/sec;
- Vertical to horizontal permeability ratio = $K_V / K_H = 0.2$.

Total porosity (TP) and effective porosity (EP) are also required for the model. Initial estimates of TP and EP were obtained from literature. Parameters used in the development of the model were:

- TP = 0.3
- EP = 0.3

9.6 **Recharge and Evapotranspiration**

Rainfall and runoff are the major components of aquifer recharge. As the runoff component is difficult to estimate and assumed to be minor compared to rainfall infiltration. As such, runoff was not included in the groundwater model.

The site is located near to Taree Airport AWS (060141) weather station. According to data from Bureau of Meteorology (BOM) for this weather station between 1997 to November 2024:

- Average monthly temperatures vary between 6.7°C (July) to 29.0°C (January).
- Average monthly rainfall varies between 195 mm/month (March) and 44 mm/month (August).
- Mean annual rainfall was reported as 1154 mm/year with a maximum annual rainfall of 1970 mm/year (2022).

Evapotranspiration (ET) data was obtained from BOM for Taree Airport AWS (060141) for a period of about four years (January 2020 to November 2024). Average annual ET was reported to be 1181 mm/year with a maximum ET of 1223 mm/year (2023).

The mean annual rainfall was utilised in the model, then calibrated to adjust the percentage of rainfall entering the model to observed groundwater level in wells. Calibrated recharges percentages are shown in Table 11 and initial evapotranspiration parameters are shown in Table 12.



Table 11: Pre-development recharge parameters

Mean annual rainfall	Calibrated recharge (% rainfall)			
(Taree Airport AWS 1997-2024) (mm/year)	Undeveloped areas	Developed areas		
1154	20	10		

Table 12: Pre-development evapotranspiration parameters

Mean evapotranspiration	Evapotranspi	Extinction donth		
(Taree Airport AWS 2020-2024) (mm/year)	Undeveloped areas	Developed areas	Extinction depth (m)	
1181	50	50	2	

9.7 Visual MODFLOW Flex settings

The following settings were selected in Visual MODFLOW Flex version 10 to best simulate conditions expected at the site:

- Flow Type: Saturated (Constant Density); and
- Numeric Engine: USGS MODFLOW 2005.

9.8 **Pre-development groundwater model**

Calibration of a flow model refers to the trial and error process by which model parameters are adjusted to produce an acceptable match between simulated and observed groundwater levels (during the monitoring period).

The steady state calibration of three cases was aimed at reproducing the observed groundwater levels in groundwater monitoring wells on 5 November 2024 and expected flow patterns in the geometry of the model. Comparison of observed water level and modelled water level are shown below in Table 13.



Well	Observed water level (AHD) (05/11/2024)	Modelled water level (AHD)	Difference in calibration			
MW1	0.97	1.20	+0.23			
MW3	0.72	0.73	+0.01			
MW4	0.64	0.79	+0.15			
101	1.15	1.27	+0.12			
102	0.88	0.90	+0.03			
103	0.64	0.50	-0.13			
104	0.61	0.39	-0.21			
105	1.03	1.29	+0.27			
	Average					

Table 13: Calibration of model

Overall, the model calibration provides an acceptable representation of the groundwater system and is considered suitable for the purposes of assessing the sensitivity of the groundwater system to changed induced by the proposed development.

It is a noted that a steady state calibration is non-unique and that there is a range of possible combinations of hydraulic conductivity vs recharge which would produce a similar calibration. In reality the hydraulic conductivity and therefore calibrated recharge could vary by about 50% to 200%. The hydraulic conductivity adopted is expected to be moderately conservate and therefore actual hydraulic conductivity more likely to be on the higher side than lower. For the purposes of assessment potential mounding due to the development each matching pair or recharge/hydraulic permeability would be expected to result in similar results, with higher permeability having slightly less mounding.

The modelled groundwater levels for pre-development scenario are shown in Figure 13 below.



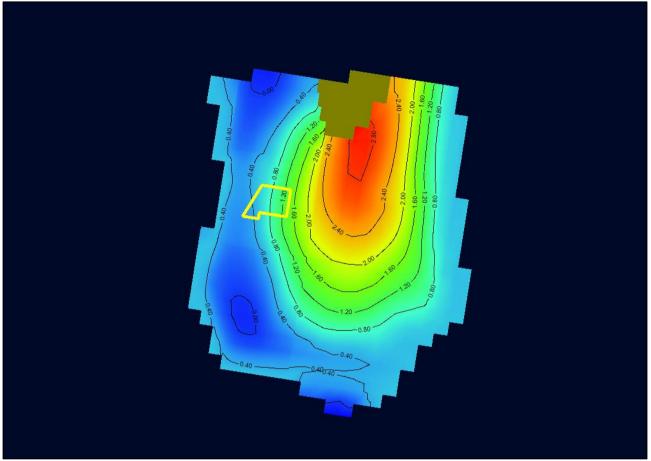


Figure 13: Pre-development – modelled groundwater elevation, 0.4 m contours, site boundary shown in yellow outline

9.9 **Post-development groundwater model**

Modelling the proposed development is based on pre-development model, with some changes relevant to proposed development. Reference to provided Stormwater Plans (ADWJ drawing 190835-32-WCMP-001 to 009 revision A dated 29/11/2024), it is understood that portions of the site are 0%, 50%, 75% and 90% impervious.

To model the post-development scenario, the following changes were made (relative to predevelopment scenario):

- Surface contour at the site changed to filled levels based on supplied DEM;
- Recharge at the site is modified proportional to % impervious (considering the calibrated recharge for undeveloped areas of the site was 20% rainfall);
 - 0% impervious: 20% rainfall;
 - 50% impervious: 10% rainfall;
 - 75% impervious: 5% rainfall; and
 - 90% impervious: 2% rainfall.



- Additional recharge in raingardens to account for run-off from impervious areas (see Table 14 below).
 - Catchment area, raingarden area and proposed development within each catchment is considered when calculating run-off and therefore is calculated for each Catchment separately.
 - Calculation of run-off assumes that all rainfall landing on impervious areas contributes to run-off proportional to % impervious, e.g. for 75% impervious areas, 75% of rainfall contributes to run-off and is equally distributed over raingarden areas.

Catchment	Catchment area (m²)	Total raingarden area (m²)*	Direct rainfall in basin (% rainfall)	Recharge from run-off (% rainfall)
1	44000	697		4275
2	73000	1411	20	2950
3	20000	490	20	2675
4	26300	665		2575

Table 14: Post-Development Recharge in Raingardens

Notes to table:

RL Reduced level (m AHD)

The modelled groundwater levels for pre-development scenario are shown in Figure 14 below and compared to pre-development scenario in Table 15.

^{*} Sum of Raingarden A and Raingarden B



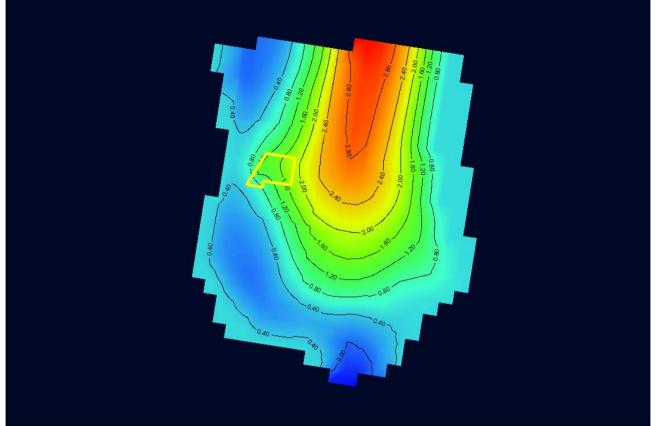


Figure 14: Post-development – modelled groundwater elevation, 0.4 m contours, site boundary shown in yellow outline

Location	Pre	Post	Predicted mounding (m)				
Location	development	development	Base case	Wet case	Dry case		
MW1	1.20	1.79	0.59	0.65	0.48		
MW3	0.73	1.48	0.75	0.87	0.58		
MW4	0.79	1.44	0.65	0.74	0.52		
101	1.27	1.80	0.53	0.60	0.43		
102	0.90	1.38	0.48	0.57	0.38		
103	0.50	1.16	0.66	0.78	0.51		
104	0.39	1.06	0.67	0.80	0.51		
105	1.29	1.82	0.53	0.57	0.45		

Table 15: Summary of modelling outputs

Change of groundwater levels was also estimated at distances from the edge of the proposed development. The estimated change of groundwater level is summarised below in Table 16.



boundary	Change of water level between Pre and Post(m)Dry caseBase caseWet case0.3 to 0.60.4 to 0.70.4 to 0.8		
Distance from site (m)	Change of water level between Pre and Post(m)		
Distance from site (m)	Dry case	Base case	Wet case
25	0.3 to 0.6	0.4 to 0.7	0.4 to 0.8
50	0.3 to 0.5	0.3 to 0.6	0.4 to 0.7
100	0.2 to 0.4	0.3 to 0.5	0.3 to 0.6
200	0.1 to 0.3	0.2 to 0.4	0.2 to 0.4

Table 16: Comparison of the change of modelled groundwater level at distances from site boundary

9.10 Limitations of modelling

The precision of the results presented in Table 15 and Table 16 are not indicative of the accuracy of the modelling, the precision provided to allow comparison between pre and post development cases, with the actual accuracy less than the precision shown.

It is considered that the modelling undertaken provides a reasonable representation of the average groundwater flow conditions occurring on the site for the periods of the modelling and provides an indication of the likely average groundwater mounding which may occur below the site and surrounds following development. It is noted that the model does not account for transient effects and mounding that will occur in the shorter term following specific rainfall events, a transient model would be required to assess this. Similarly, localised effects in the immediate vicinity of any infiltration areas may be greater than presented in Table 15.

Changes in longer term climatic conditions will have impacts on overall groundwater levels across the site and surrounds and this would include potential climate change effects. Any future sea level changes would have a commensurate impact on groundwater level changes.

10. Estimated Groundwater Impacts

10.1 **Proposed development impacts**

Based on Douglas' understanding of the proposed development plans, the anticipated changes in site recharge conditions have been used to predict approximate average groundwater mounding below and around the site.

The groundwater modelling predicts mounding of up to about 0.8 m may occur under the site under average rainfall conditions.

These estimates are reliant on the type of fill placed and assume that sand filling will be used, and all low permeability being stripped from the site prior to filling. For low permeability fill the degree of mounding would be expected to be less due to reduced recharge and for intermediate permeability such as a silty sand the mounding could be higher.



The most likely time for mounding to occur within the fill is during construction before impervious surfaces and surface drainage has been installed, however, this is highly weather dependant. Mounding in the filling is only expected to be an issue when the water table comes within the depth of influence of building foundations, pavements and other infrastructure. Once the site is fully developed mounding is expected to occur in the vicinity of infiltration areas following rainfall events.

Higher mounding within the fill can be expected to occur in the following scenarios:

- During and following above average rainfall.
- During placement of filling and construction prior to construction of impervious areas
- In the vicinity of infiltration areas, in particular during and following specific rainfall events.

Short term mounding within the filling has some potential to impact on surface infrastructure such as building footings and road pavement and can be managed by appropriate installation of subsurface drainage with the fill.

A number of measures can be used to limit the propensity for mounding in the fill:

- Removal of all low permeability layers from the surface prior to placement of filling at the site. This is especially important if higher permeability filling is to be placed over. The need for removal of such material will depend on its lateral extent and what other measures are proposed to limit water levels as outlined below;
- Placement of higher permeability material in the lower parts of the filling, with any lower permeability material used at higher levels to form a capping layer;
- Installation of appropriate subsoil drainage about 1.0 to 1.5 m below the finished ground surface to protect pavements and building footings from saturation under more severe conditions;
- If fill other than sand with minimal fines is used, then careful consideration should be given to the layering of such soils to prevent placing lower permeability soils below permeable soils. Lower permeability soils, if used, will reduce the effectiveness of subsoil drainage and are preferably placed near the upper sections of fill to provide a capping above drainage measures.

Douglas has not specifically modelled the impacts of climate change which may have an overall impact on groundwater levels across the region commensurate with any changes in sea level that may occur in the future as well as potential changes in rainfall patterns. Development impacts are generally expected to be incremental to the background conditions and therefore modelling of climate change scenarios is not considered necessary to assess the incremental impacts from the proposed development.

10.2 Off site impacts

Modelling predicts that average groundwater levels will be elevated within the vicinity of the site due to the groundwater mounding. Under wet conditions the degree of mounding is predicted to be higher and lower under dry conditions.

Modelling predicts that groundwater dependent ecosystems near to the site will experience a general elevation of water table due to the proposed development.



The results of the groundwater model indicate a general elevation of the groundwater table in the vicinity of the site, particularly in wetter conditions, however, these average levels are within the seasonal variations observed by groundwater monitoring to date.

Given that some areas to the south and west of the site exhibit natural surface ponding of groundwater from time to time, this ponding can be expected to occur more often and to some increased extent under seasonally wetter conditions. The groundwater model is sensitive to regional permeability of subsurface soils. A moderately conservative permeability has been utilised in the groundwater model therefore the mounding may be less than those predicted by the groundwater model (i.e. higher regional permeability would result in a reduction of groundwater mounding).

11. Conclusions

The results of the groundwater assessment indicate that there is potential for increases in groundwater levels below the site and surrounds within.

Groundwater monitoring to date has shown these levels can come close to the surface following above average rainfall and this has the potential to affect surface infrastructure. Recommendations are provided to manage groundwater mounding within the site.

The predicted average level of mounding is within seasonal variations of groundwater level and reduces with distance from the site. Therefore negative impacts to groundwater level from the proposed development are expected to be limited. Additional surface ponding and groundwater seepage to surface drainage will likely occur on the surrounds of the site. Reduced average water levels potentially affecting GDEs or existing registered wells are not expected to occur.

The degree of groundwater mounding has been assessed against average groundwater level and average rainfall conditions over one month of groundwater monitoring and is therefore approximate only. Majority of the groundwater recharge is expected following large storm events and will result in a greater degree of mounding for these events due to above average recharge. A transient model would be required to quantify short term mounding.

Ongoing groundwater monitoring would be helpful to further establish baseline conditions and monitor potential post development impacts.

12. **References**

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13. Limitations

Douglas Partners Pty Ltd (Douglas) has prepared this report for this project at 40-80 and 82 Chapmans Road, Tuncurry with reference to Douglas' proposal 219536.00.P.005.Rev0 dated 26 November 2024 and acceptance received from Stephanie Vanderent dated 26 November 2024. The work was carried out under Douglas' Engagement Terms. This report is provided for the exclusive use of Allam MHE Developments No. 2 Pty Ltd for this project only and for the purposes as described in the report. It should not be used by or relied upon for other projects or purposes on the same or other site or by a third party. Any party so relying upon this report beyond its exclusive use and purpose as stated above, and without the express written consent of Douglas, does so entirely at its own risk and without recourse to Douglas for any loss or damage. In preparing this report Douglas has necessarily relied upon information provided by the client and/or their agents.

The results provided in the report are indicative of the sub-surface conditions on the site only at the specific sampling and/or testing locations, and then only to the depths investigated and at the time the work was carried out. Sub-surface conditions can change abruptly due to variable geological processes and also as a result of human influences. Such changes may occur after Douglas' field testing has been completed.

Douglas' advice is based upon the conditions encountered during this investigation. The accuracy of the advice provided by Douglas in this report may be affected by undetected variations in ground conditions across the site between and beyond the sampling and/or testing locations. The advice may also be limited by budget constraints imposed by others or by site accessibility.



The assessment of atypical safety hazards arising from this advice is restricted to the groundwater components set out in this report and based on known project conditions and stated design advice and assumptions. While some recommendations for safe controls may be provided, detailed 'safety in design' assessment is outside the current scope of this report and requires additional project data and assessment.

This report must be read in conjunction with all of the attached and should be kept in its entirety without separation of individual pages or sections. Douglas cannot be held responsible for interpretations or conclusions made by others unless they are supported by an expressed statement, interpretation, outcome or conclusion stated in this report.

This report, or sections from this report, should not be used as part of a specification for a project, without review and agreement by Douglas. This is because this report has been written as advice and opinion rather than instructions for construction.

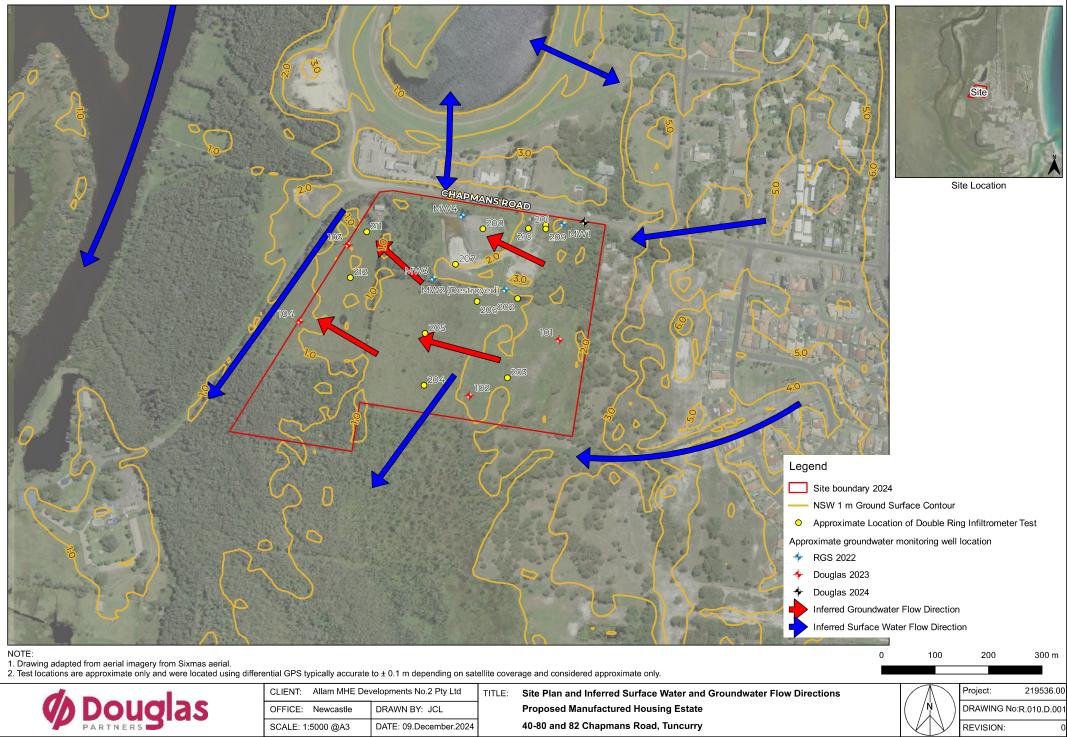
Appendix A

Drawing 1 – Site Plan and Inferred Surface Water and Groundwater Flow Directions

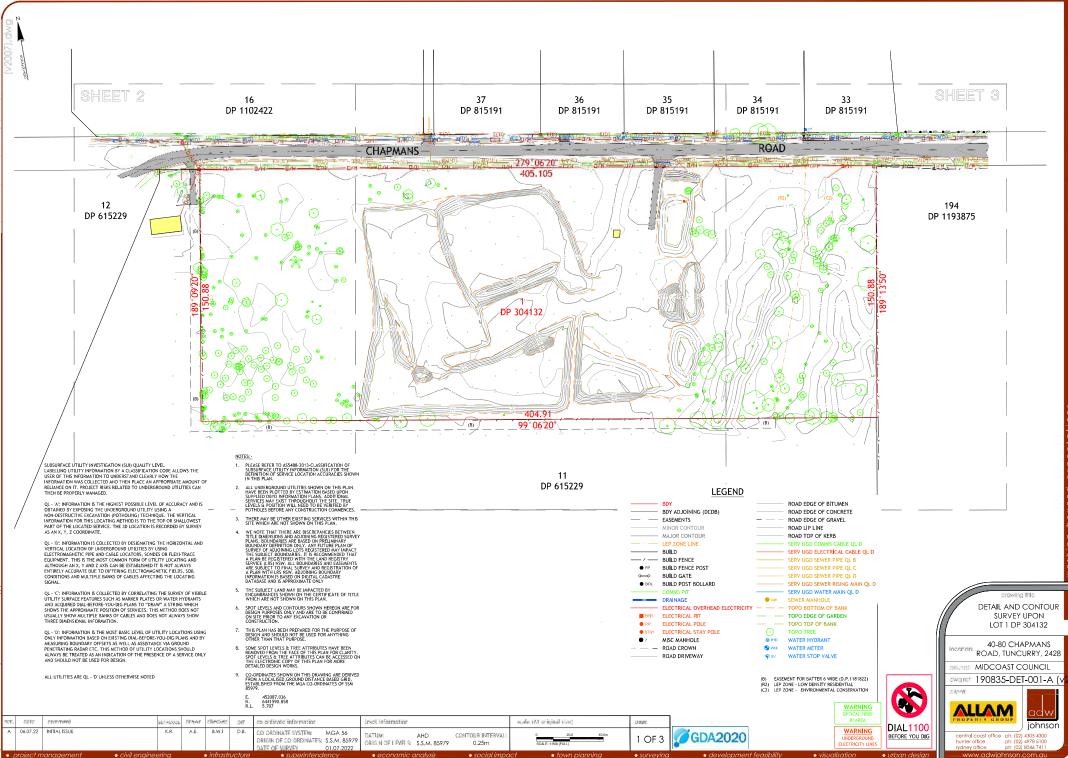
Detailed Survey – ADW Johnson – 190835-DET-001-A Revision A

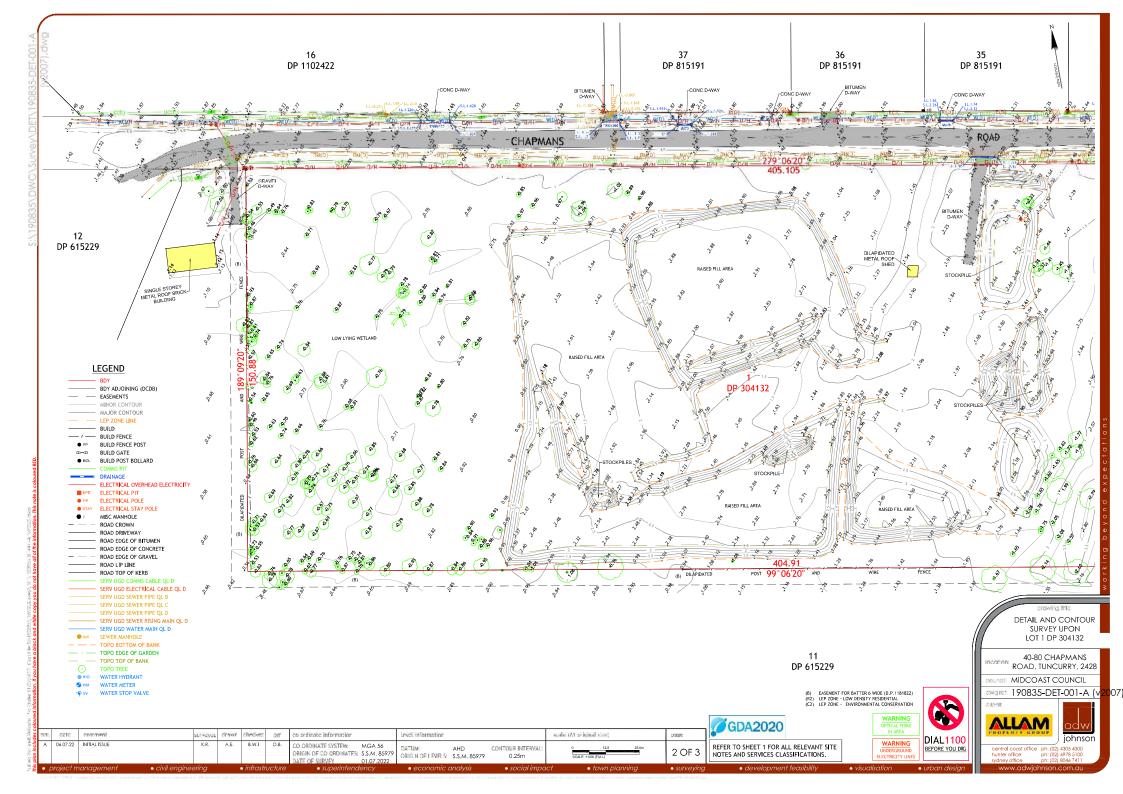
Concept Engineer Plans (ADW Johnson - 190835 -CENG – Revision A dated 09/12/2024

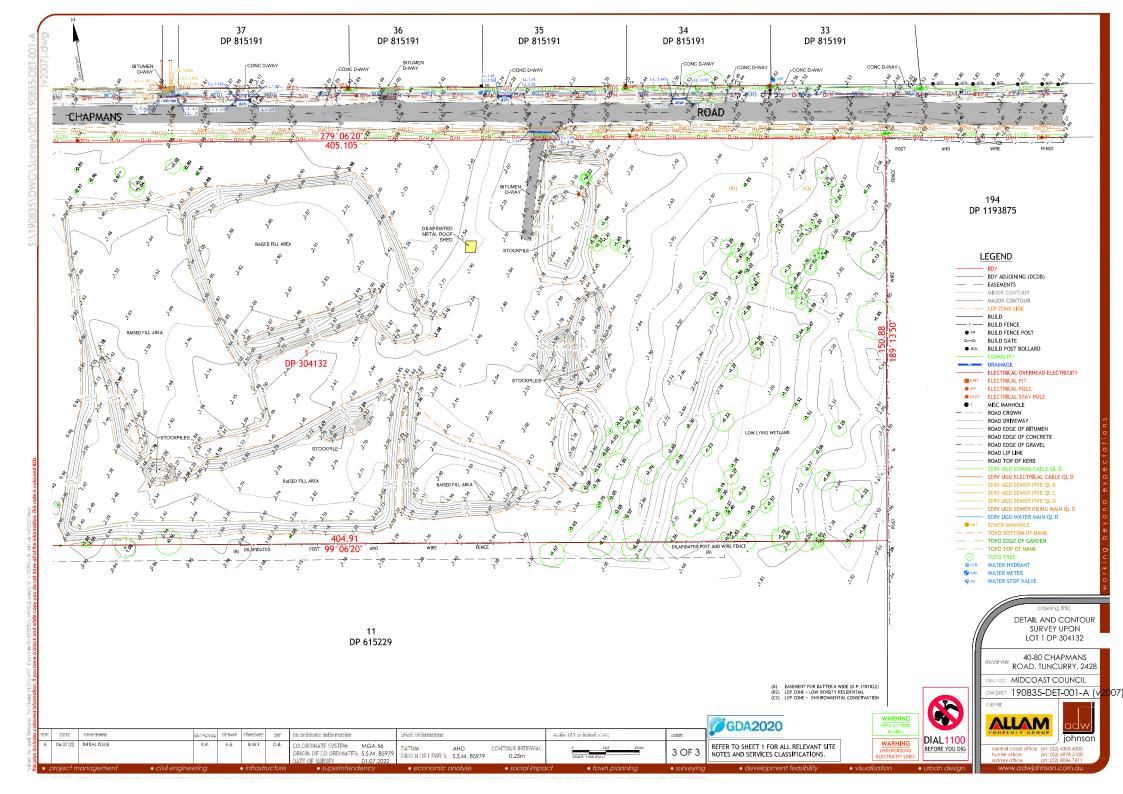
Master Plan – ADW Johnson – 190835 – MP – Revision K dated 09/12/2024



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CONCEPT ENGINEERING PLANS OF ' PROPOSED MANUFACTURED HOME ESTATE ' LOT 100, D.P. 1286524 & LOT 11, D.P.615229 40-80, 82 CHAPMANS ROAD, TUNCURRY

90835-S2-CENG-00



LOCALITY SKETCH

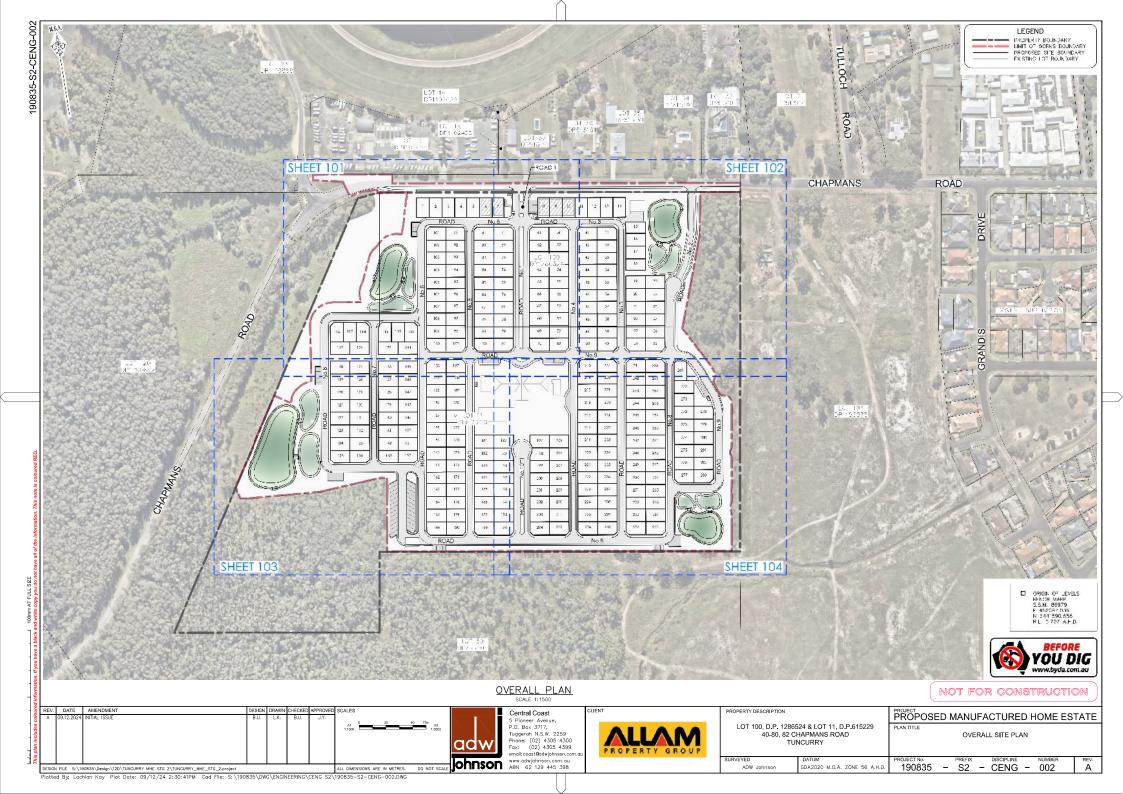
	INDEX OF DRAWINGS
DRAWING No.	DRAWING NAME
190835-S2-CENG-001 190835-S2-CENG-002 190835-S2-CENG-003	COVER SHEET, LOCALITY SKETCH & INDEX OF DRAWINGS OVERALL SITE PLAN SITE DEMOLITION & TREE CLEARING PLAN
190835-S2-CENG-101 190835-S2-CENG-102 190835-S2-CENG-103 190835-S2-CENG-104 190835-S2-CENG-111	DETAIL PLAN - SHEET 1 DETAIL PLAN - SHEET 2 DETAIL PLAN - SHEET 3 DETAIL PLAN - SHEET 4 SITE ENTRY DETAIL PLAN
190835-S2-CENG-201 190835-S2-CENG-211 190835-S2-CENG-212 190835-S2-CENG-213 190835-S2-CENG-215 190835-S2-CENG-215 190835-S2-CENG-216 190835-S2-CENG-218 190835-S2-CENG-219 190835-S2-CENG-219	TYPICAL ROAD CROSS SECTIONS & DETAILS ROAD LONGITUDINAL SECTION - CHAPMANS ROAD ROAD LONGITUDINAL SECTION - ROAD No.1A & 1B ROAD LONGITUDINAL SECTION - ROAD No.2 ROAD LONGITUDINAL SECTION - ROAD No.3 ROAD LONGITUDINAL SECTION - ROAD No.4 ROAD LONGITUDINAL SECTION - ROAD No.6 ROAD LONGITUDINAL SECTION - ROAD No.6 ROAD LONGITUDINAL SECTION - ROAD No.7 ROAD LONGITUDINAL SECTION - ROAD No.8 ROAD LONGITUDINAL SECTION - ROAD No.8 SHEET 1 ROAD LONGITUDINAL SECTION - ROAD NO.8 SHEET 2
190835-S2-CENG-221 190835-S2-CENG-222 190835-S2-CENG-223	ROAD LONGITUDINAL SECTION - ROAD No.9 ROAD LONGITUDINAL SECTION - ROAD No.10A & 10B ROAD LONGITUDINAL SECTION - ROAD No.11 & 12
190835-S2-CENG-301 190835-S2-CENG-302 190835-S2-CENG-303 190835-S2-CENG-304	VEHICLE SWEPT PATHS PLAN - SHEET 1 VEHICLE SWEPT PATHS PLAN - SHEET 2 VEHICLE SWEPT PATHS PLAN - SHEET 3 VEHICLE SWEPT PATHS PLAN - SHEET 4
190835-S2-CENG-401 190835-S2-CENG-411 190835-S2-CENG-412 190835-S2-CENG-413 190835-S2-CENG-414 190835-S2-CENG-415	STORMWATER LAYOUT PLAN BASIN No.1 DETAIL PLAN BASIN No.2 DETAIL PLAN BASIN No.3 DETAIL PLAN BASIN No.4 DETAIL PLAN BASIN & RAINGARDEN TYPICAL SECTION

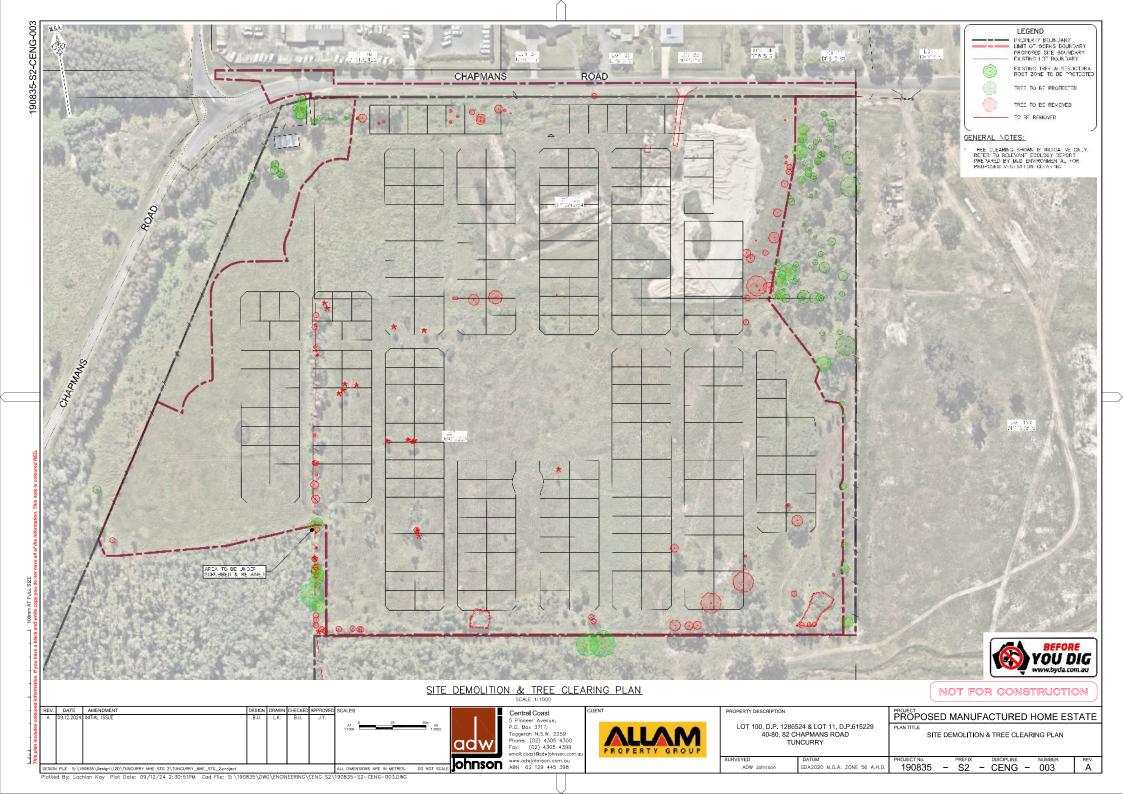
INDEX OF DRAWINGS

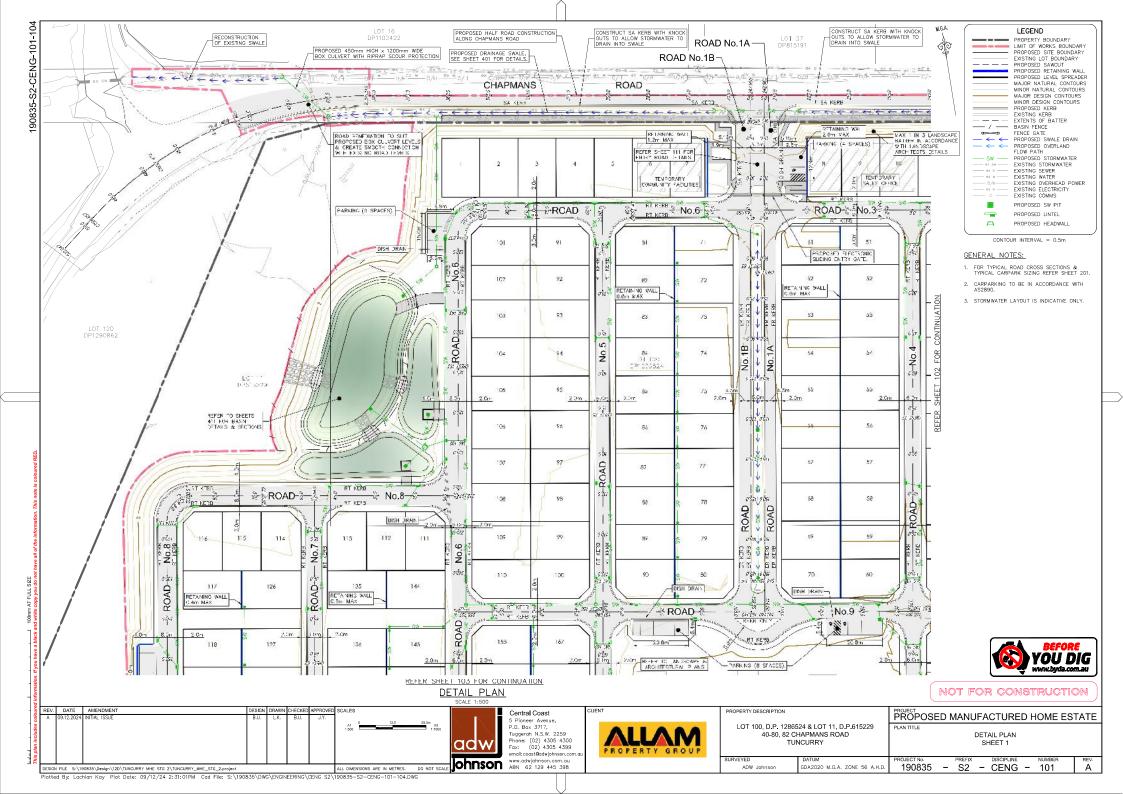
DRAWING No.	DRAWING NAME
190835-S2-CENG-501	SITE REGRADE PLAN
190835-S2-CENG-511	SITE REGRADE SECTIONS - SHEET 1
90835-S2-CENG-512	SITE REGRADE SECTIONS - SHEET 2
90835-S2-CENG-513	SITE REGRADE SECTIONS - SHEET 3
90835-S2-CENG-514	SITE REGRADE SECTIONS - SHEET 4
90835-S2-CENG-515	SITE REGRADE SECTIONS - SHEET 5
90835-S2-CENG-516	SITE REGRADE SECTIONS - SHEET 6
90835-S2-CENG-517	SITE REGRADE SECTIONS - SHEET 7
190835-S2-CENG-601	EROSION & SEDIMENT CONTROL PLAN
190835-S2-CENG-611	EROSION & SEDIMENT CONTROL DETAILS & NOTES
190835-S2-CENG-701	TRAFFIC MANAGEMENT PLAN - SHEET 1
90835-S2-CENG-702	TRAFFIC MANAGEMENT PLAN - SHEET 2
90835-S2-CENG-703	TRAFFIC MANAGEMENT PLAN - SHEET 3
90835-S2-CENG-704	TRAFFIC MANAGEMENT PLAN - SHEET 4
190835-S2-CENG-801	COMBINED SERVICES PLAN - SHEET 1
90835-S2-CENG-802	COMBINED SERVICES PLAN - SHEET 2
90835-S2-CENG-803	COMBINED SERVICES PLAN - SHEET 3
90835-S2-CENG-804	COMBINED SERVICES PLAN - SHEET 4

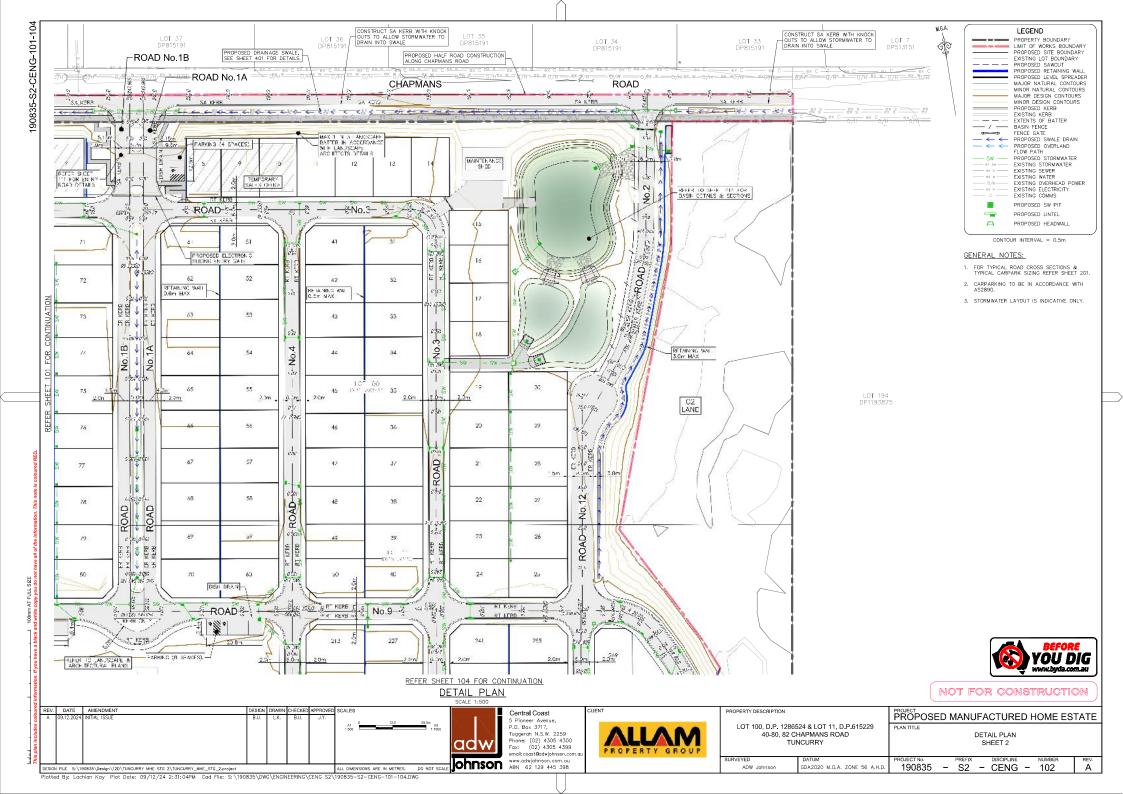


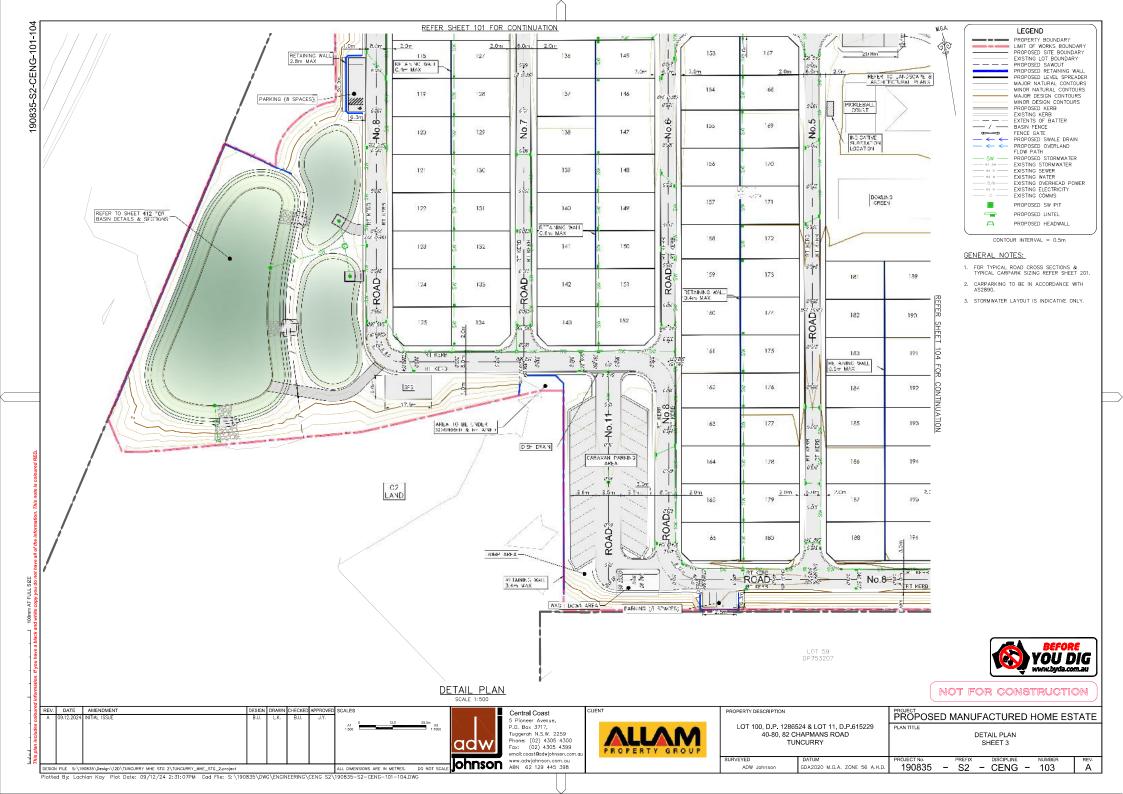
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s plan includes										P.O. E Tugge Phone Fax:	 Verifies Verifies<		LOT 100, D.P. 1286524 & LOT 11, D.P.615229 40-80, 82 CHAPMANS ROAD TUNCURRY		PLAN TITLE COVER SHEET, INDEX OF DRAWINGS & LOCALITY SKETCH				
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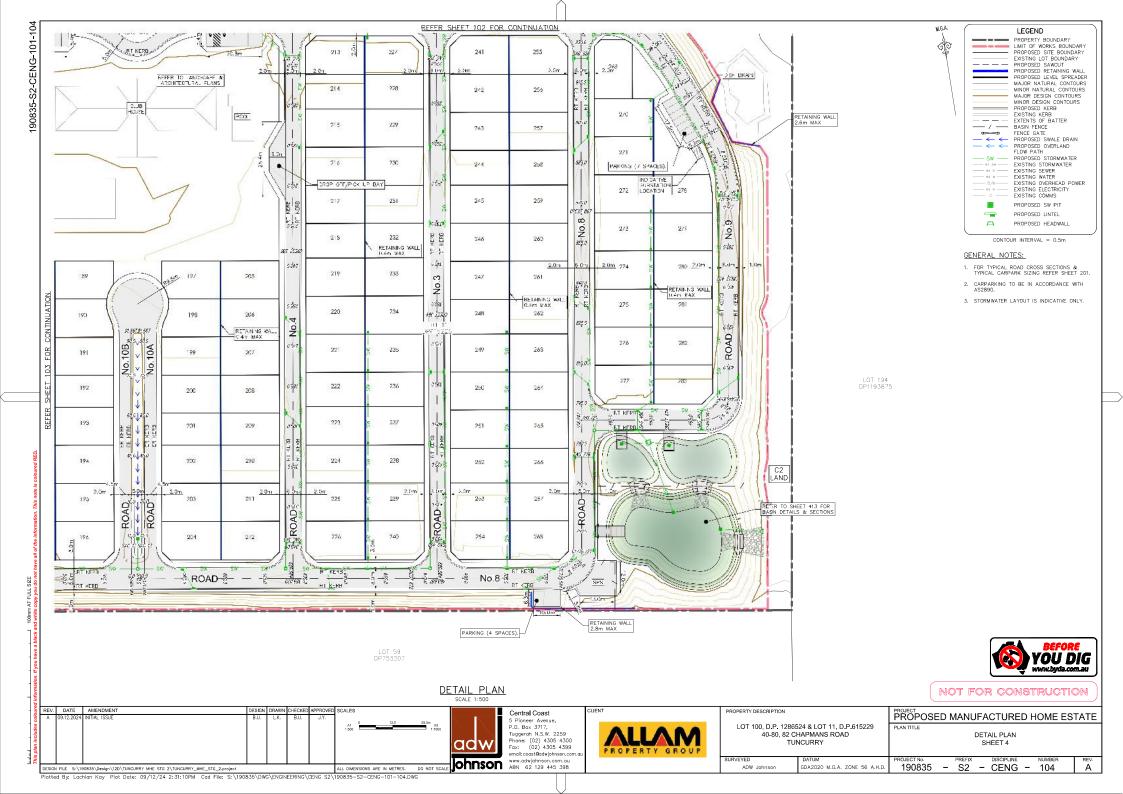


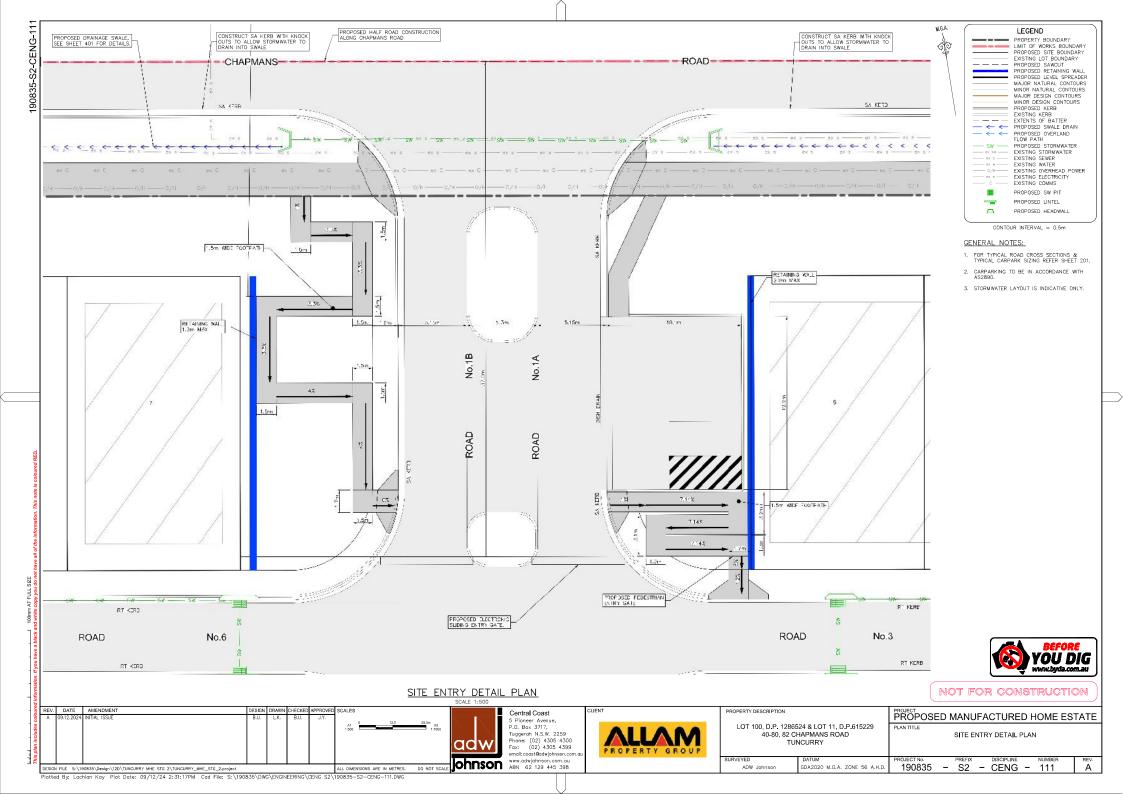


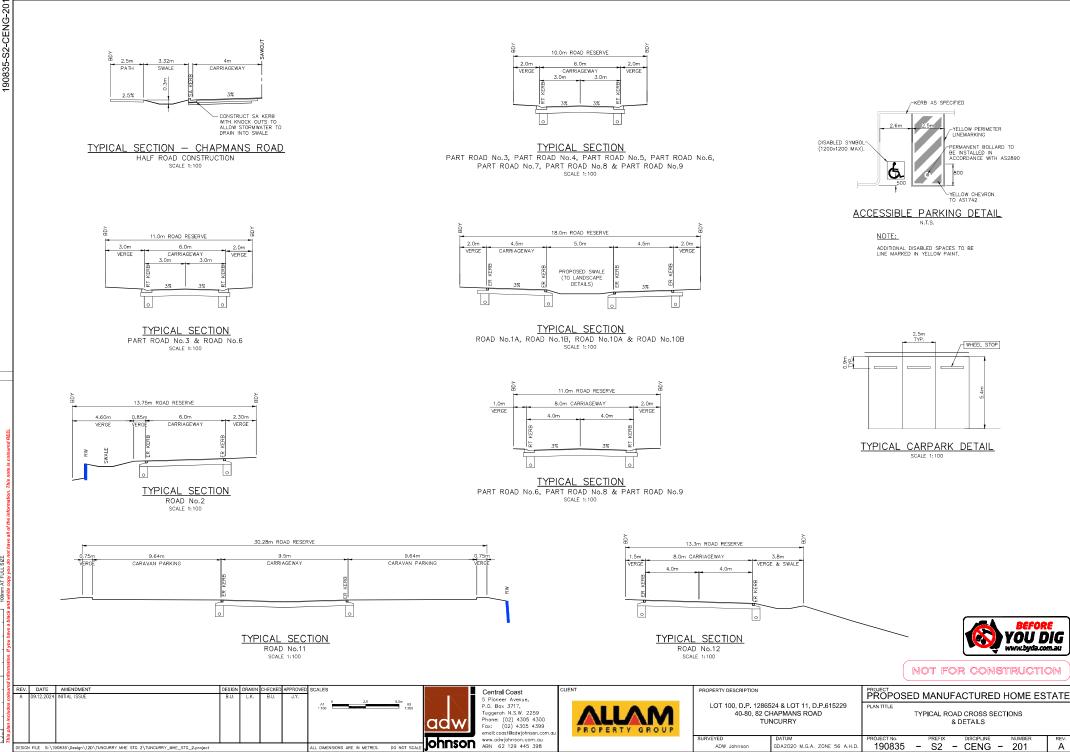






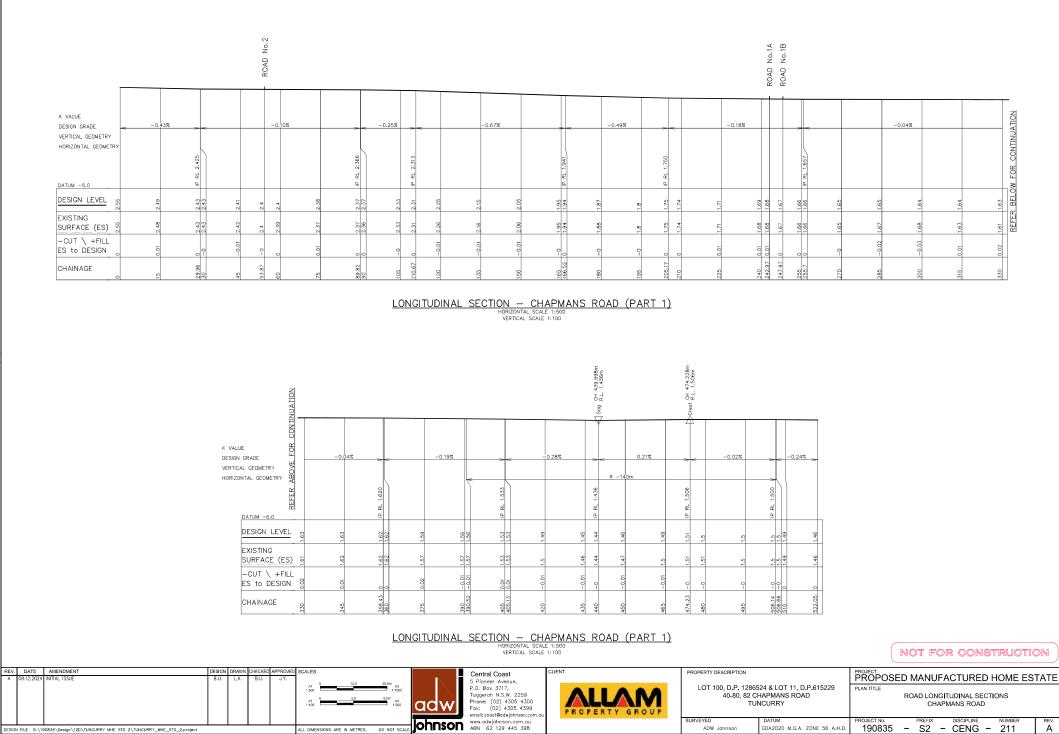






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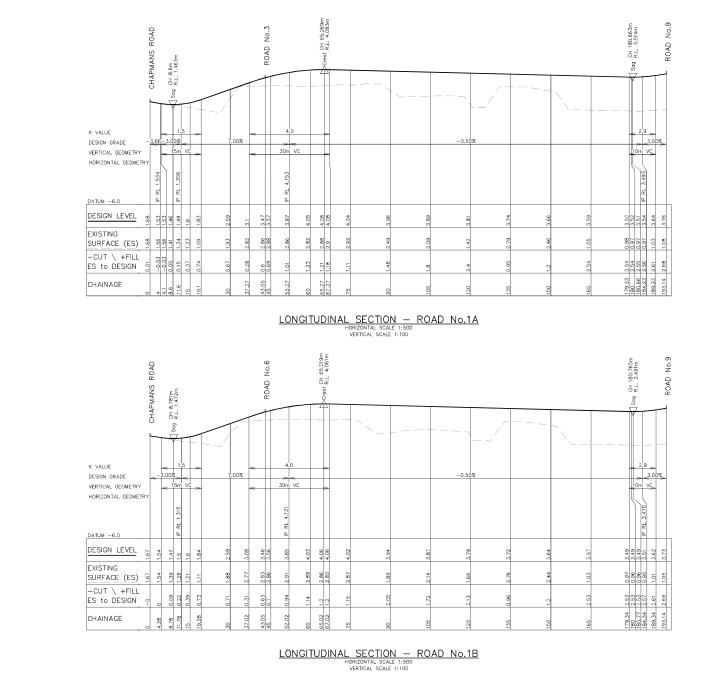
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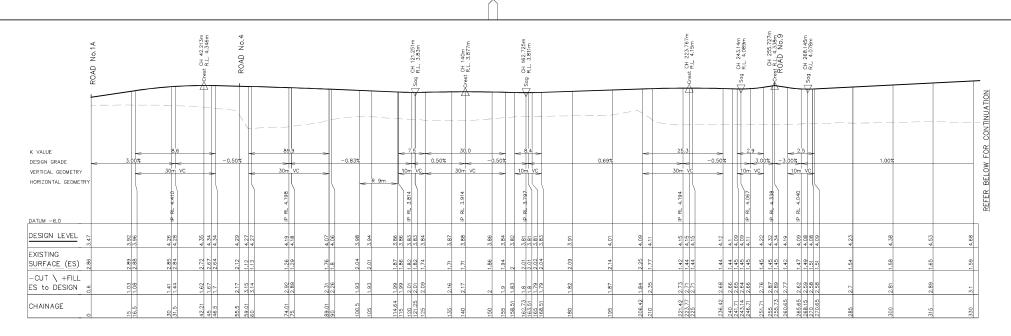
190835-S2-CENG-211

REV.



REV. DATE AMENDMENT DESIGN DRAWN CHECKED APPROVED SCALES Central Coast 5 Pioneer Avenue, PROPERTY DESCRIPTION PROPOSED MANUFACTURED HOME ESTATE LIEN ITTAL ISSUE LOT 100, D.P. 1286524 & LOT 11, D.P.615229 A1 1:80 P.O. Box 3717, Tuggerah N.S.W. 2259 PLAN TITLE 40-80, 82 CHAPMANS ROAD ROAD LONGITUDINAL SECTIONS Phone: (02) 4305 4300 Fax: (02) 4305 4399 TUNCURRY adw ROAD No.1A & 1B PROPERTY GROUP email: coast@adwjohnson.con SURVEYED ohnson www.adwjohnson.com.au ABN 62 129 445 398 REV. GDA2020 M.G.A. ZONE 56 A.H.D. 190835 - S2 - CENG - 212 DESIGN FILE S:\190835\Design\12D\TUNCURRY_MHE_STG_2\TUNCURRY_MHE_STG_2.project ALL DIMENSIONS ARE IN METRES. DO NOT SCAL ADW Johnson Plotted By: Lachlan Kay Plot Date: 09/12/24 2:31:31PM Cad File: S:\190835\DWG\ENGINEERING\CENG S2\190835-S2-CENG-212.DWG

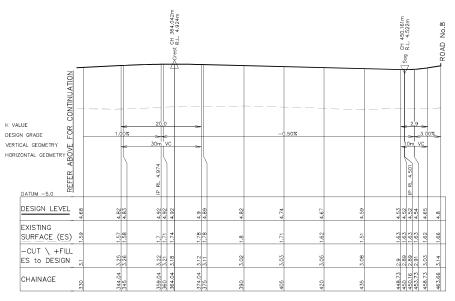
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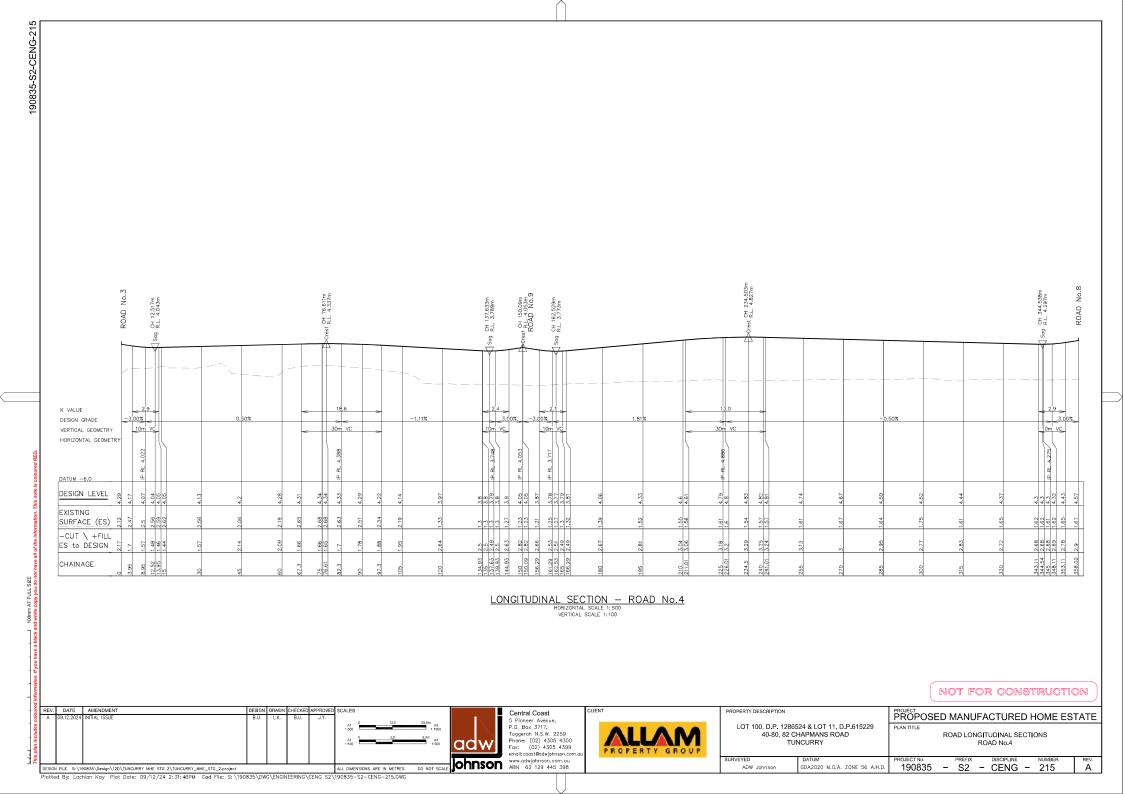
LONGITUDINAL SECTION - ROAD No.3 (PART 1) HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:100

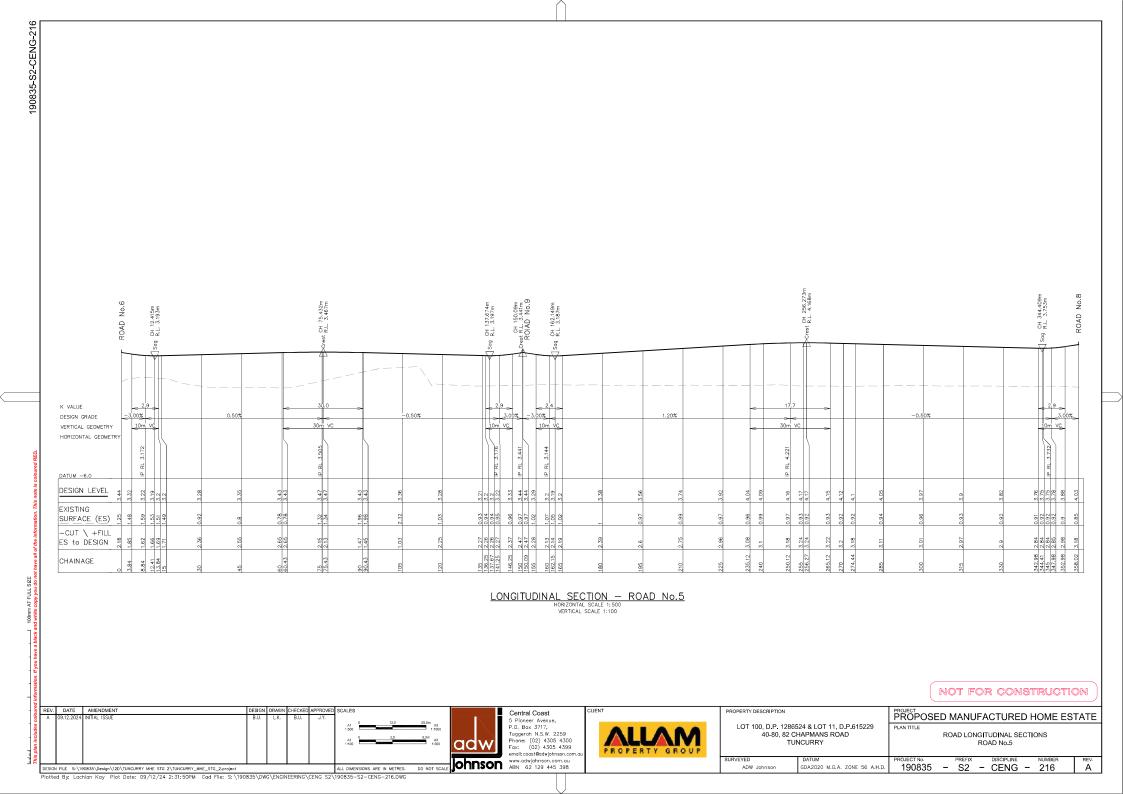




LONGITUDINAL SECTION - ROAD No.3 (PART 2) HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:100

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dinil This	DESIGN FILE S:\190835\Design\12D\TUNCURRY MHE STG 2\TUNCURRY_MHE_STG_2.project	ALL DIMENSIONS ARE IN METRES. DO NOT SCALE	ohnson www.adwjohnson.com.au ABN 62 129 445 398	SURVEYED DATUM ADW Johnson GDA2020 M.G.A. ZONE 5	A.H.D. PROJECT NO. PREFIX DISCIPLINE NUMBER REV. 190835 - S2 - CENG - 214 A
	Plotted By: Lachlan Kay Plot Date: 09/12/24 2:31:41PM Cad File: S:\190835\DWG\ENGINEERING\CENG	S2\190835-S2-CENG-214.DWG			· · · · · · · · · · · · · · · · · · ·



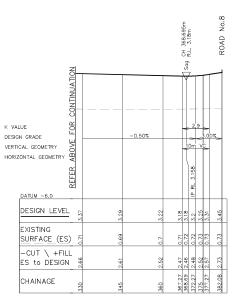


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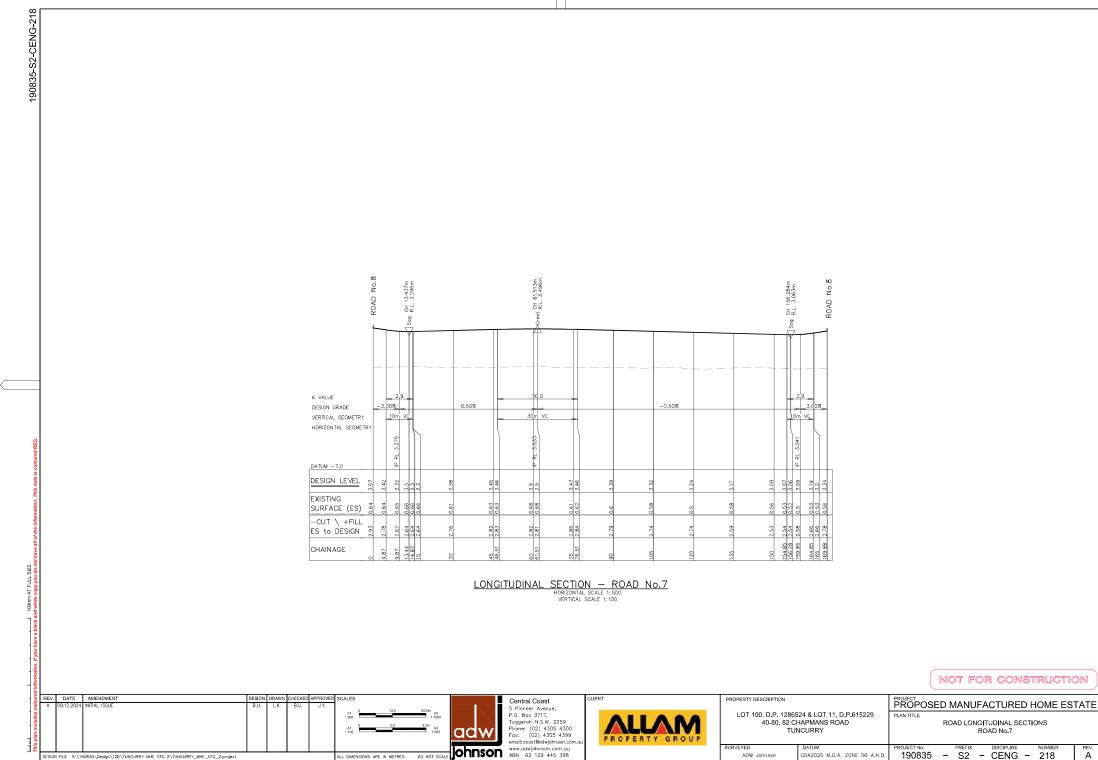
ROAD No.1B	m350g CH 17.205m		∫sag CH 82,931m L 33,13m Scast CH 102,931m Scast CH 102,931m	∫sag RL, 3.213m A. 142.931m RL, 3.213m RL, 3.213m Orest RL, 3.233m	Jsag CH 191.644m L. J.169m - ROAD No.8	- ROAD No.9	Const Cd. 282.147m 3.571m
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EXISTING SURFACE (ES)	2.51 1.51 1.45 1.52 1.53	1.46 1.48 1.48 1.39 1.39 0.71	0.71 0.7 0.65 0.65 0.67 0.67 7 7 1 2,1	0.71 0.67 0.68 0.68 0.68 0.68 0.73 0.73	0.72 0.72 0.74 0.74 0.73 0.74 0.73	0.7 0.73 0.77 0.77	0.73 0.72 0.72 0.72
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CHAINAGE	12.21 15 17.21 22.21 30	37.93 37.93 42.93 45.93 47.93 55.5 60 60	25 77.93 82.93 82.93 90 90 102.93 102.93 114.64 114.64	11.7.93 135 137.93 147.93 150 150 165 165	177.93 180.64 191.64 191.64 195.64 195.64 196.64 210 212 212 212 212 212 212 212	240 255.73 255.73 267.15 27.15	282.15 2855 300 315 330 330 330 330

LONGITUDINAL SECTION - ROAD No.6 (PART 1)



LONGITUDINAL SECTION - ROAD No.6 (PART 2)

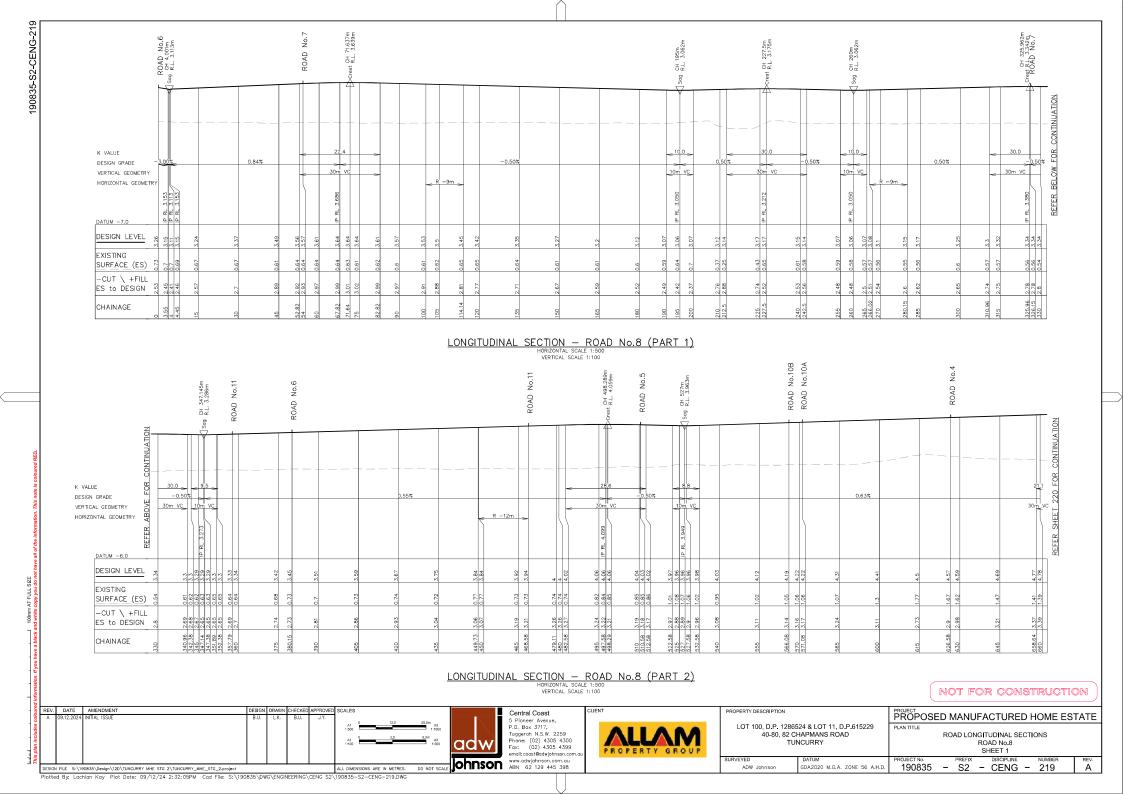
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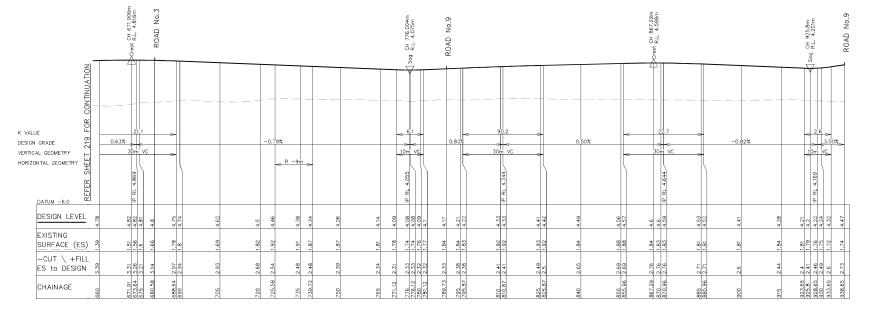
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PROJECT No. PREFIX DISCIPLINE NUMBER GDA2020 M.G.A. ZONE 56 A.H.D.

ADW Johnson

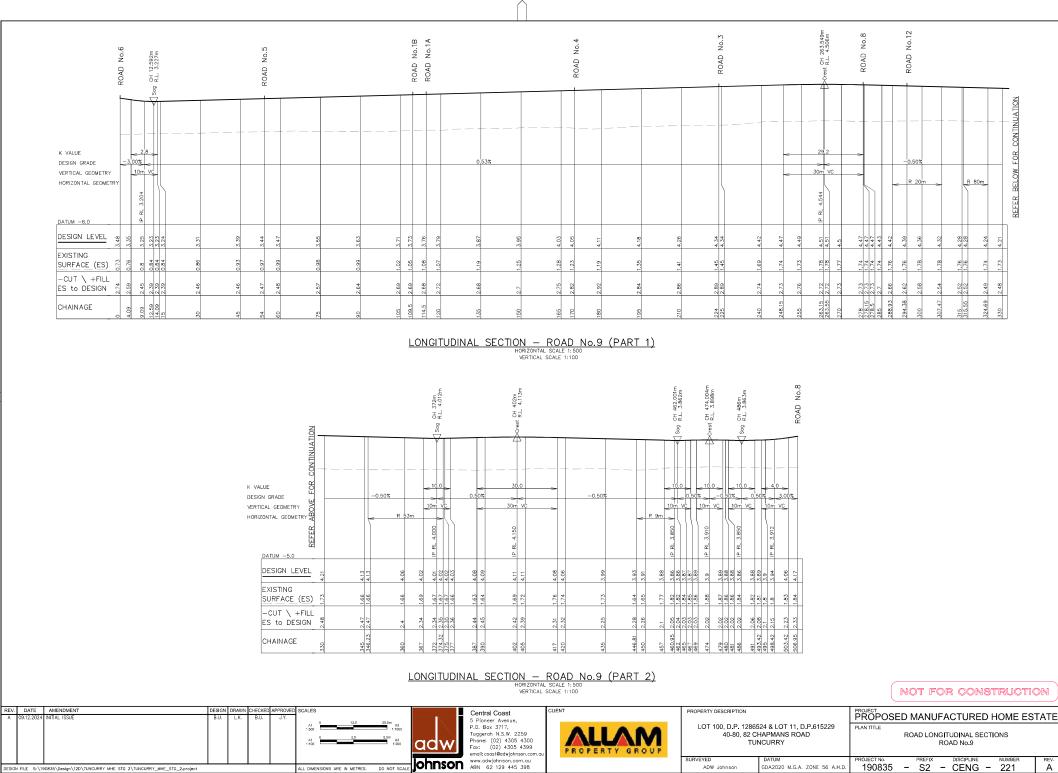


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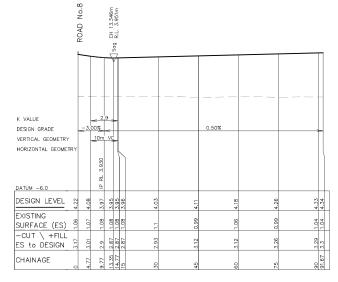
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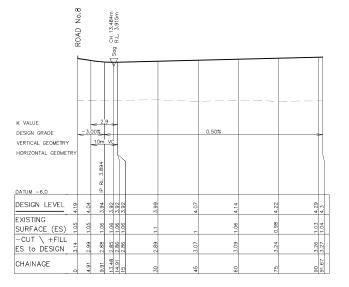


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190835-S2-CENG-221

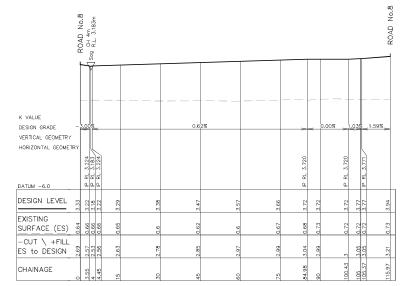


LONGITUDINAL	SECTION - ROAD No.10A	(PART 1)											
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	VERTICAL SCALE 1:100												

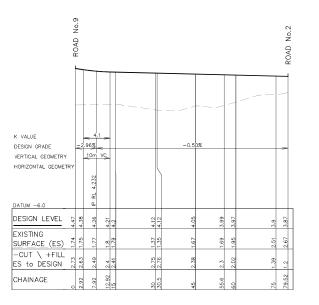


LONGITUDINAL SECTION - ROAD No.10A (PART 2) HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:100

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LONGITUDINAL SECTION - ROAD No.11 HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:100



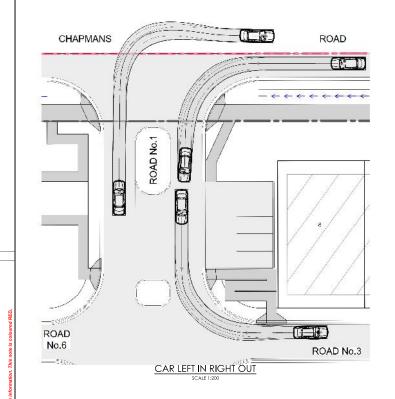
LONGITUDINAL SECTION - ROAD No.12

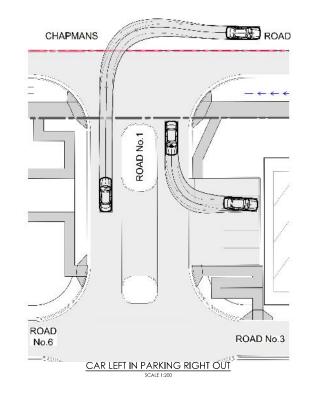
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Plotted By: Lachian Kay Plot Date: 99/12/24 2:32:25PM Cod File: S:\199835\DWG\ENGINEERING\CENG S2\199835-S2-CENG-223.DWG

190835-S2-CENG-301-304

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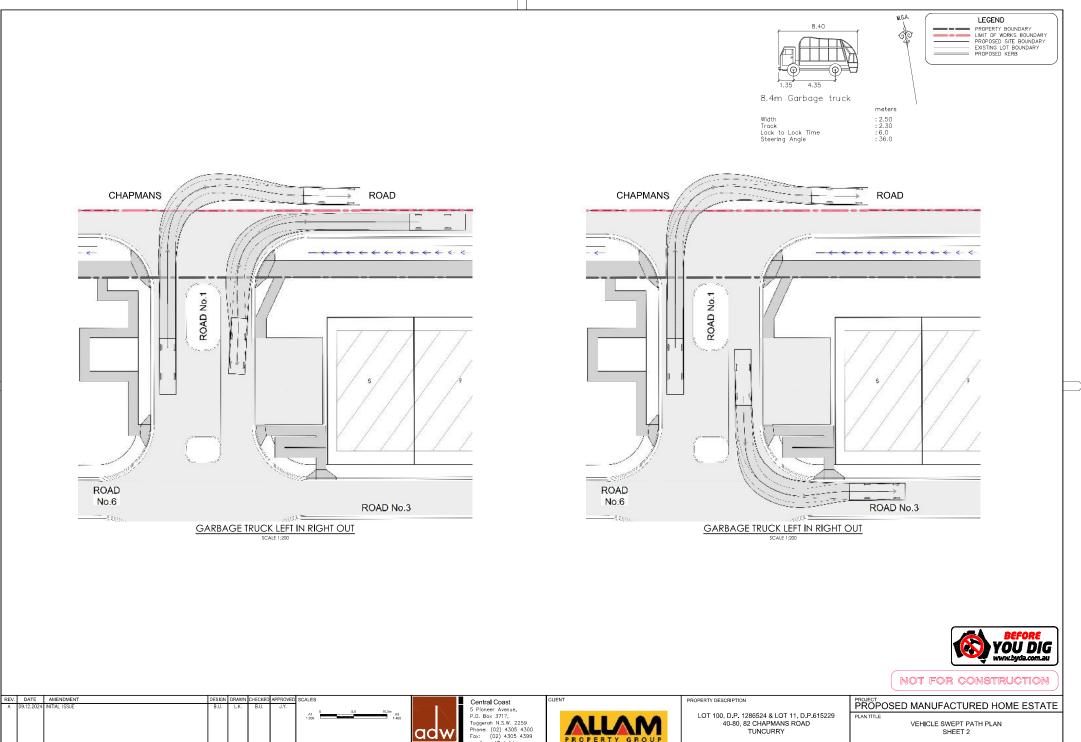




5.20 M.G.A. LEGEND PROPERTY BOUNDARY LIMIT OF WORKS BOUNDARY PROPOSED SITE BOUNDARY EXISTING LOT BOUNDARY PROPOSED KERB ්ල 0.95 3.05 B99 PASSENGER-CAR Line CA meters Width : 1.94 Track : 1.84 Lock to Lock Time : 6.0 Steering Angle : 33.6 CHAPMANS ROAD < < < ROAD No.1 H. -----ROAD ROAD No.3 No.6 244 T CAR U-TURN SCALE 1:200



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email: coast@adwjohnson.con

ohnson www.adwjohnson.com.au ABN 62 129 445 398

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190835-S2-CENG-301-304

SURVEYED GDA2020 M.G.A. ZONE 56 A.H.D. ADW Johnson

LOT 100, D.P. 1286524 & LOT 11, D.P.615229 40-80, 82 CHAPMANS ROAD TUNCURRY

PROPOSED MANUFACTURED HOME ESTATE PLAN TITLE

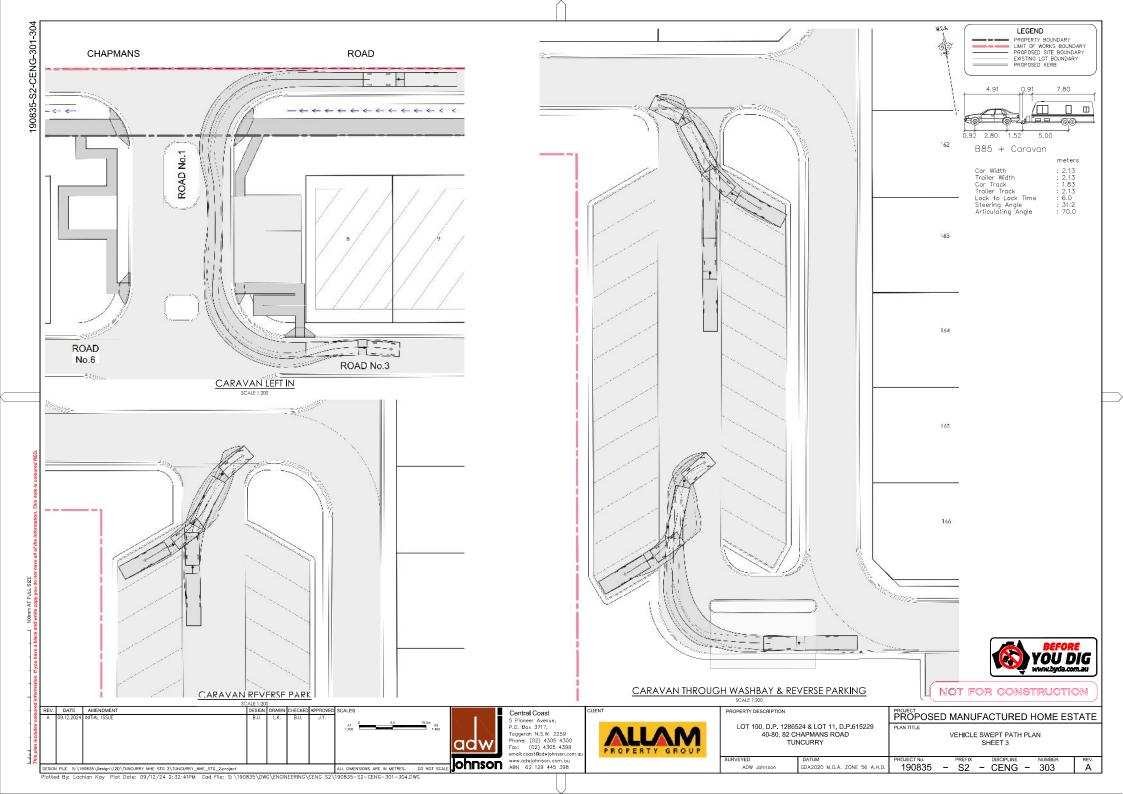
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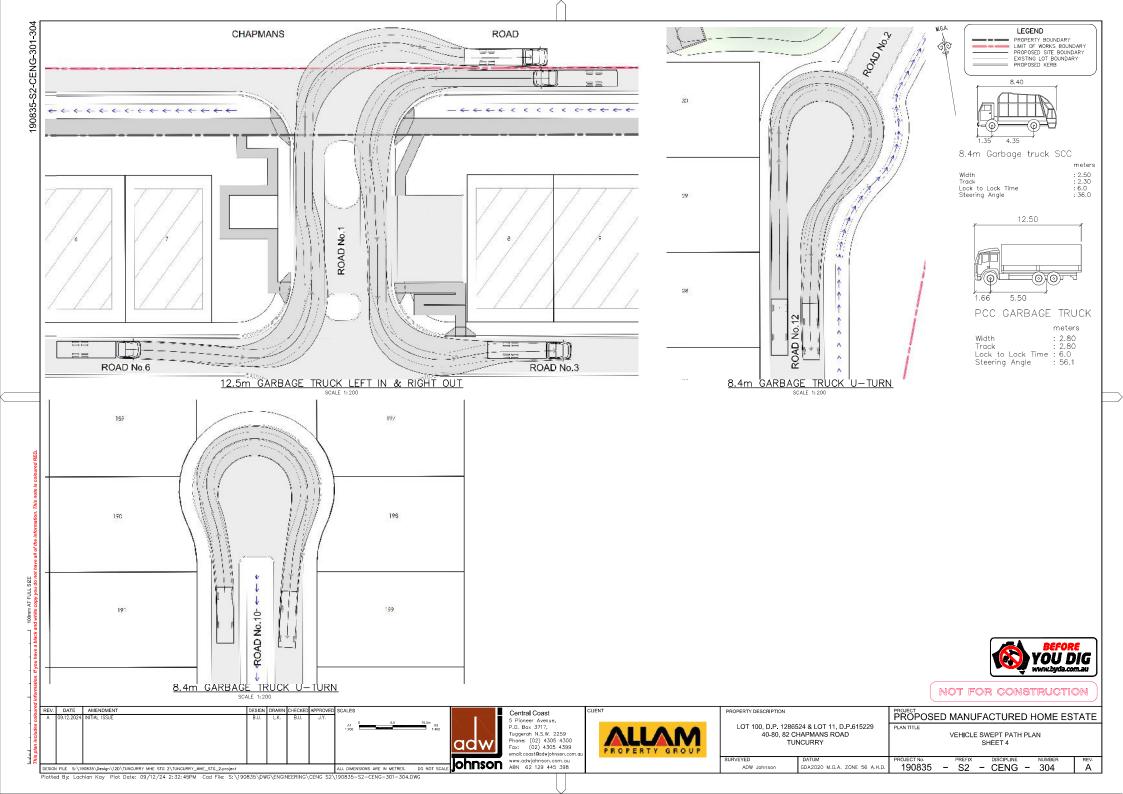
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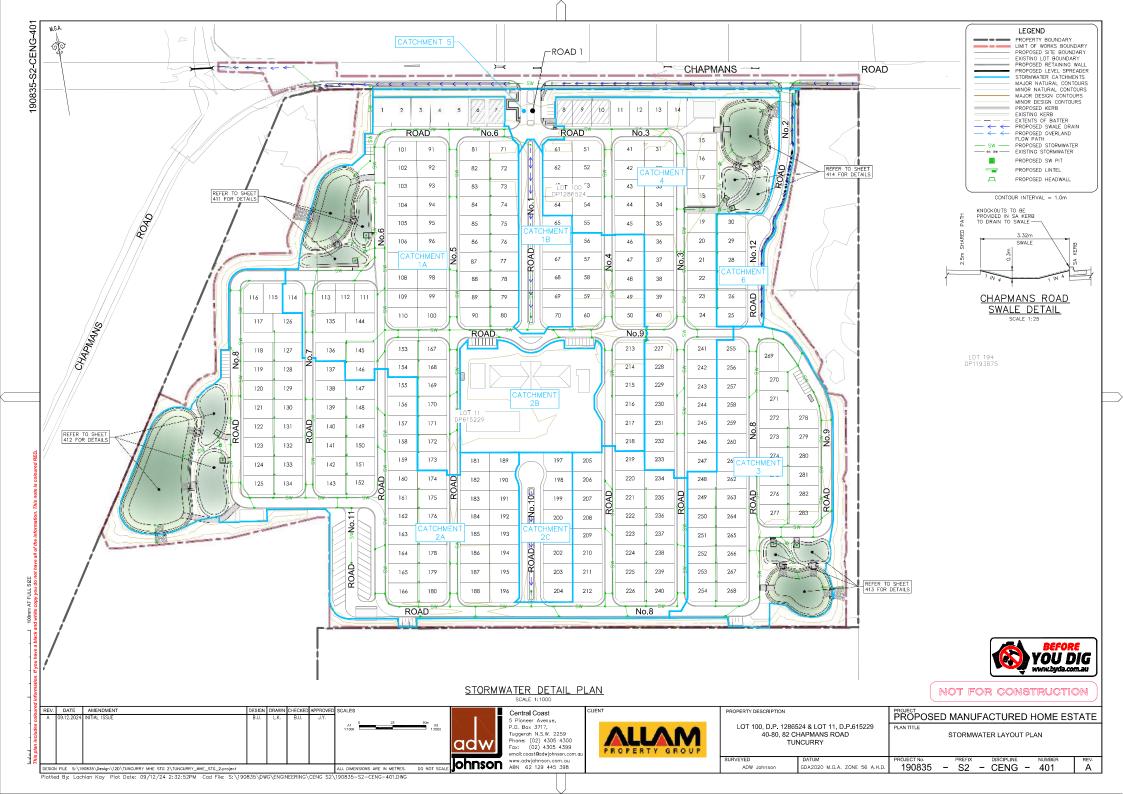
VEHICLE SWEPT PATH PLAN SHEET 2

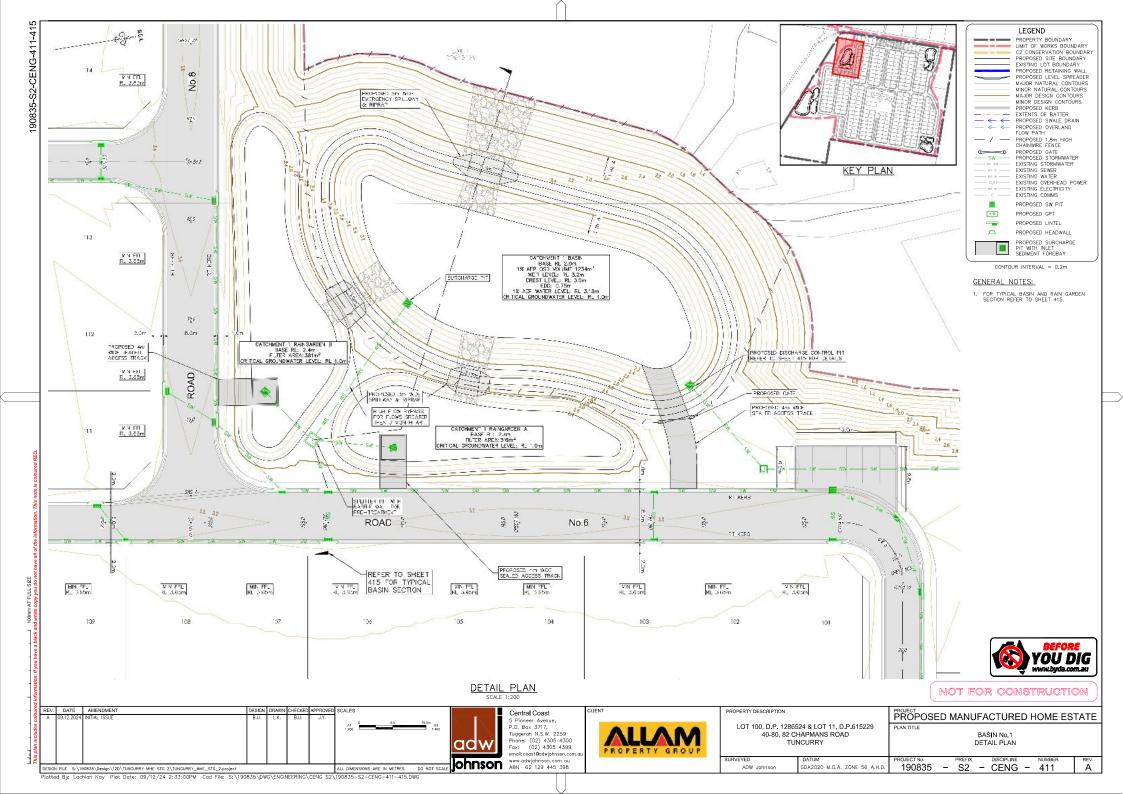
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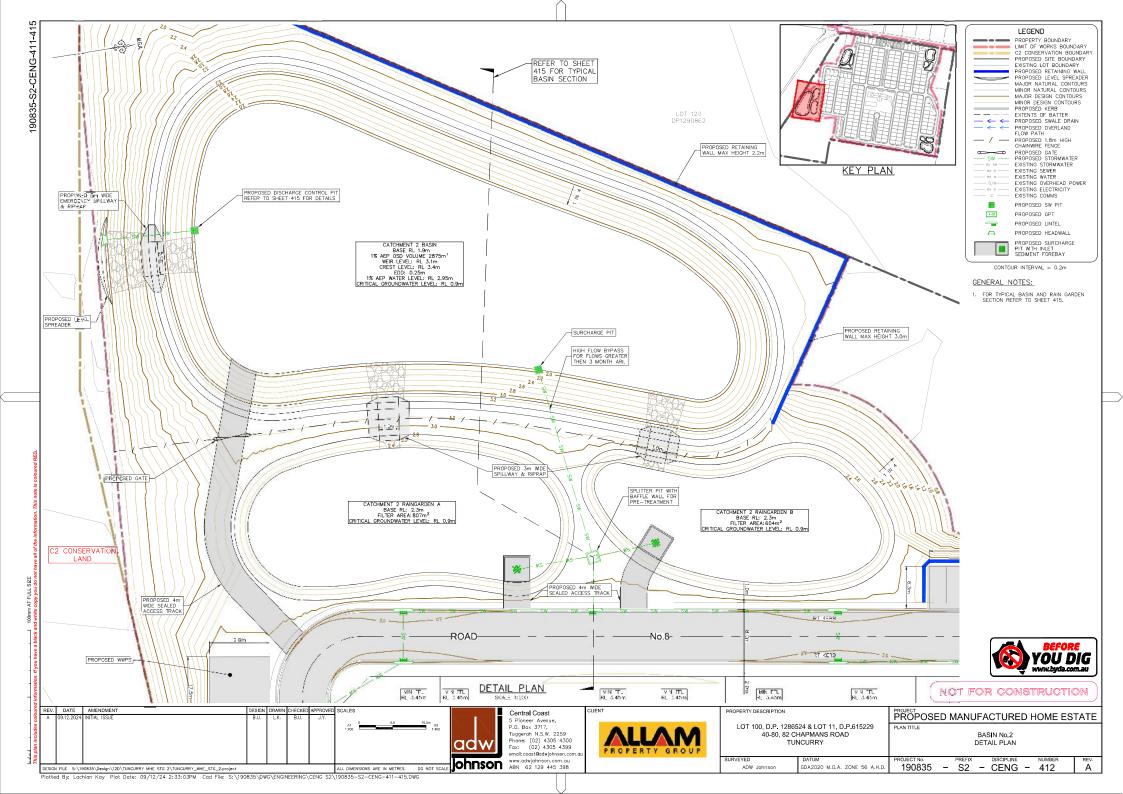
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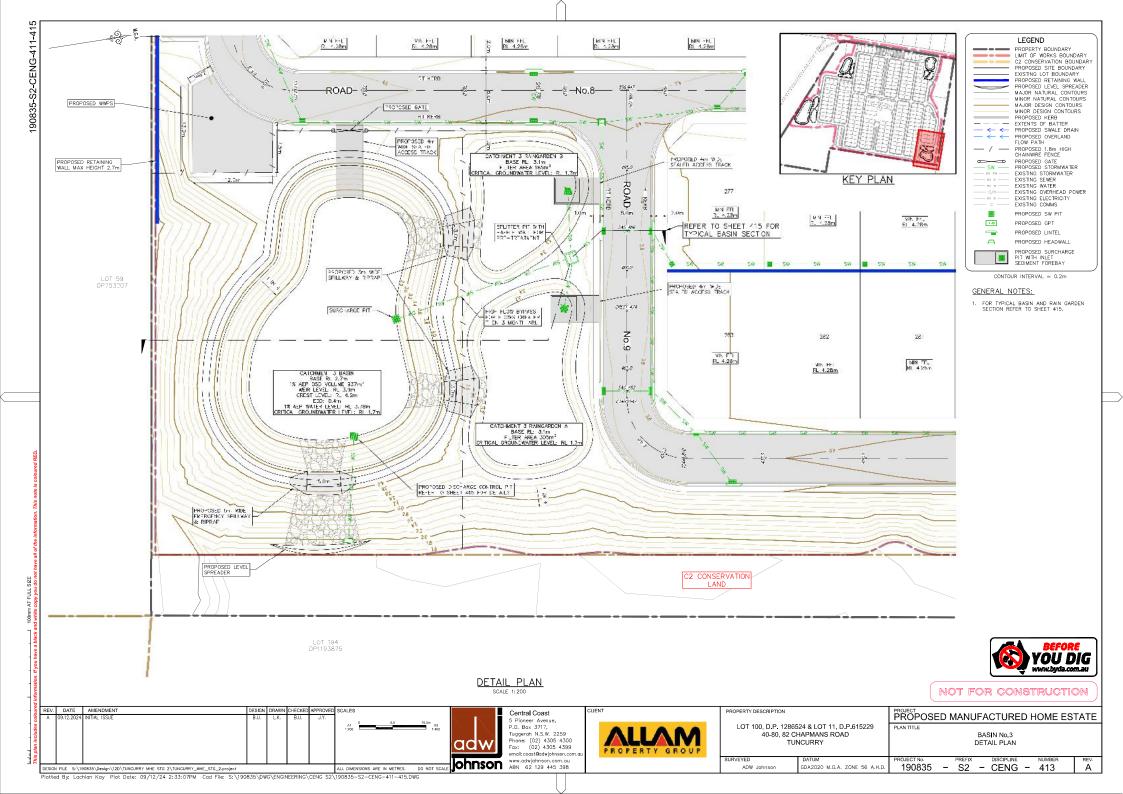


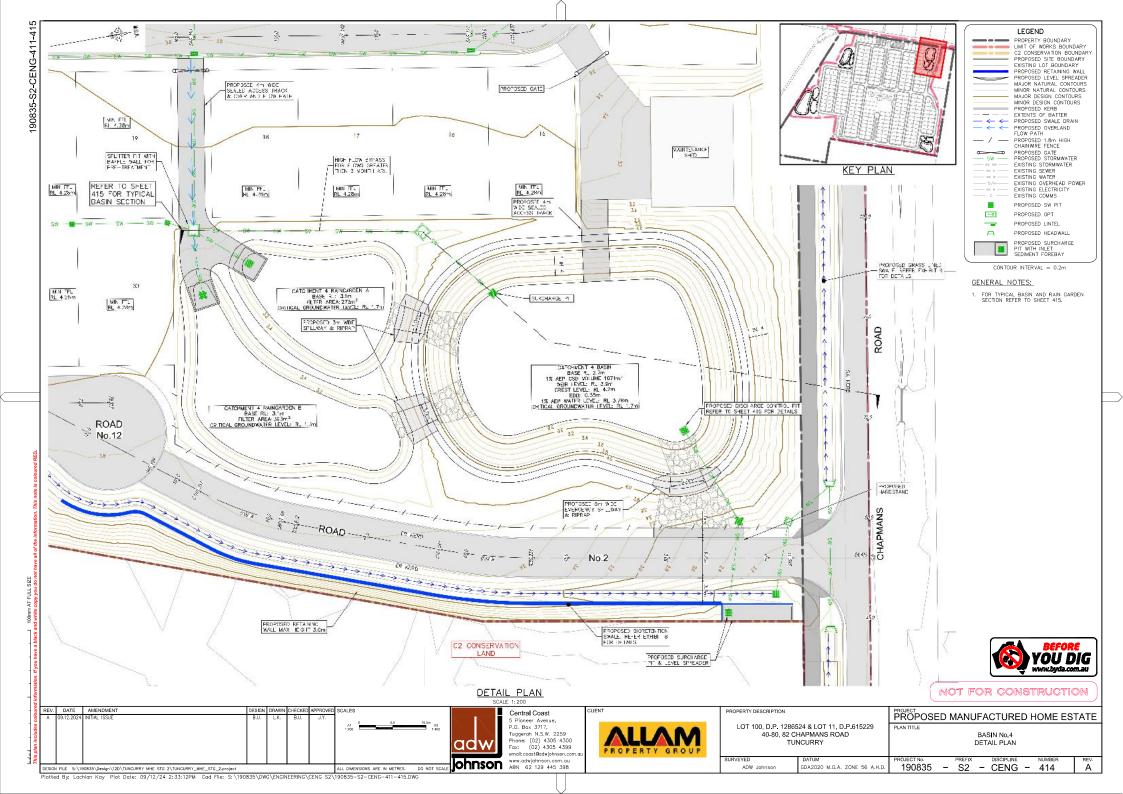


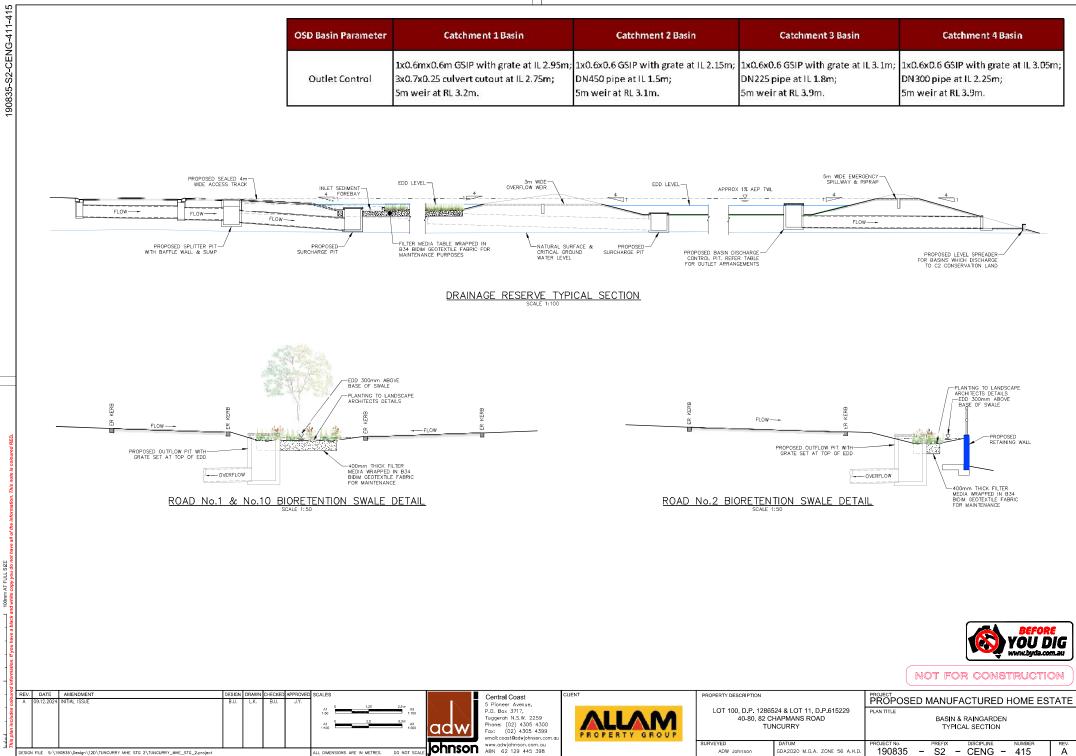








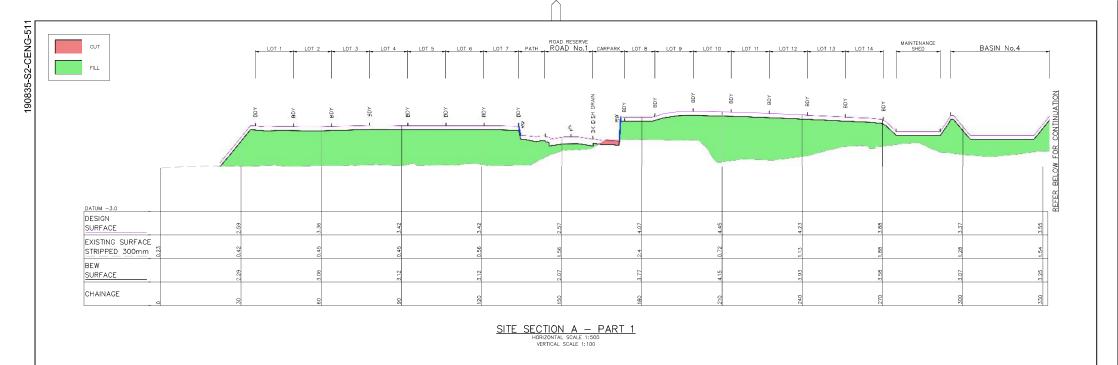


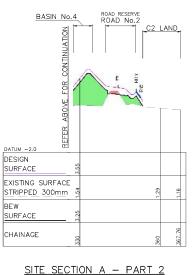


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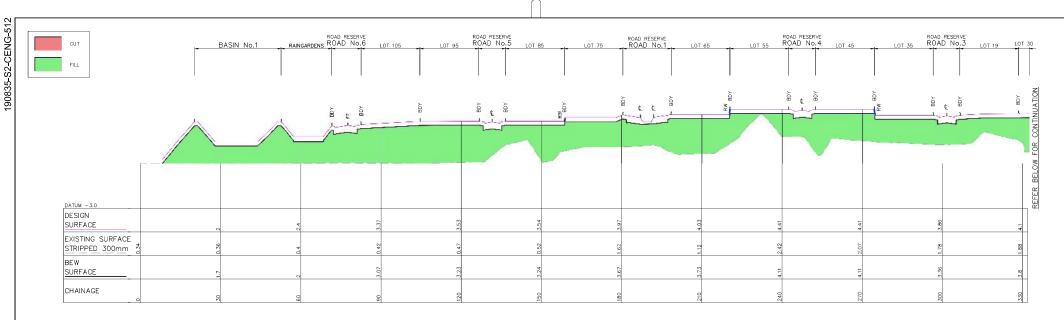
GENERAL NOTES

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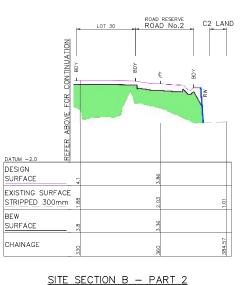




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SITE SECTION B - PART 1 HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:100





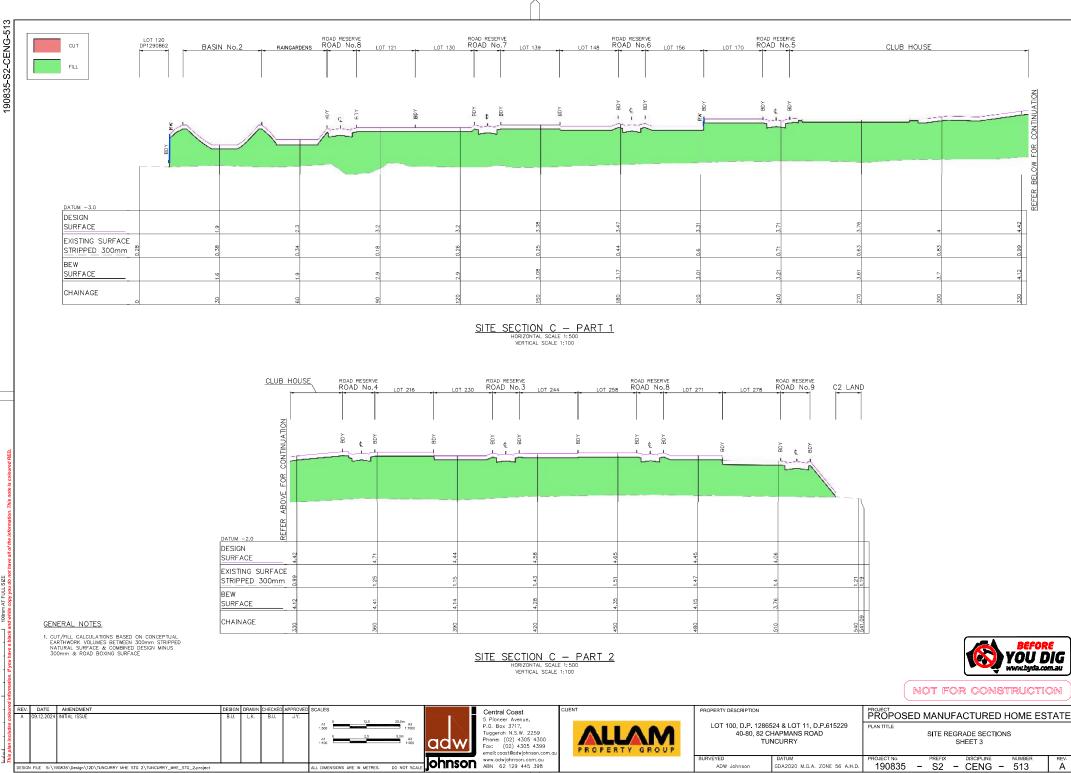
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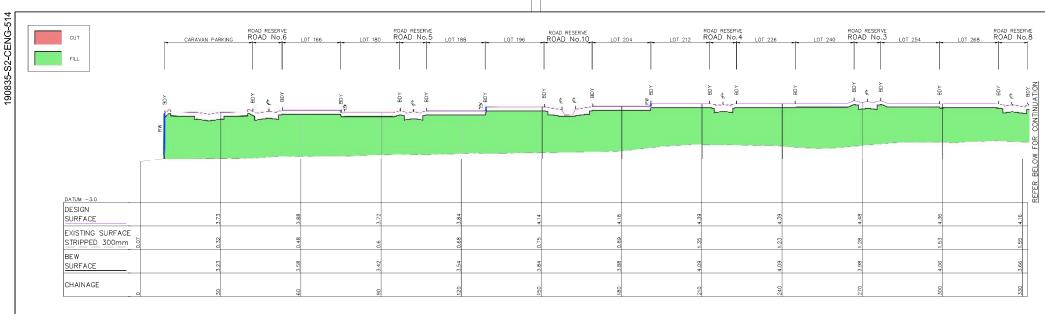
GENERAL NOTES

 CUT/FILL CALCULATIONS BASED ON CONCEPTUAL EARTHWORK VOLUMES BETMEEN 300mm STRIPPED NATURAL SURFACE & COMBINED DESIGN MINUS 300mm & ROAD BOXING SURFACE

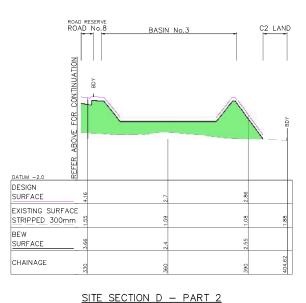


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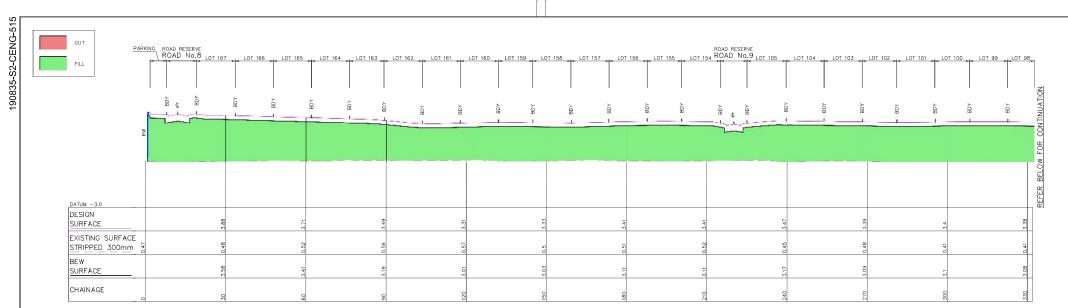
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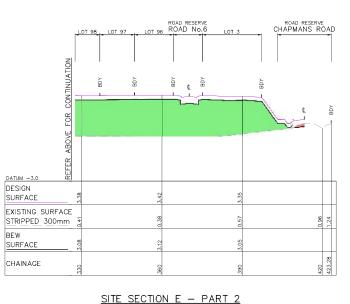
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GENERAL NOTES

 CUT/FILL CALCULATIONS BASED ON CONCEPTUAL EARTHWORK VOLUMES BETMEEN 300mm STRIPPED NATURAL SURFACE & COMBINED DESIGN MINUS 300mm & ROAD BOXING SURFACE



SITE SECTION E - PART 1 HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:100



GENERAL NOTES

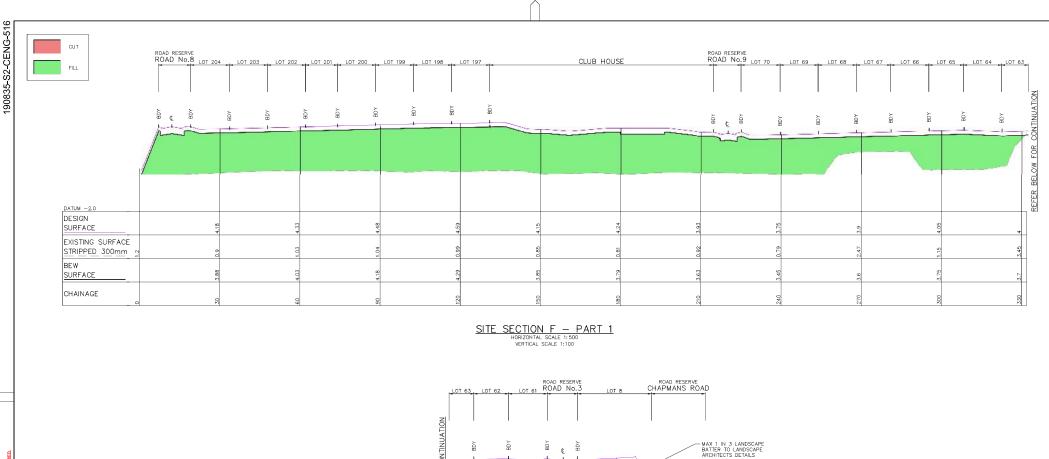
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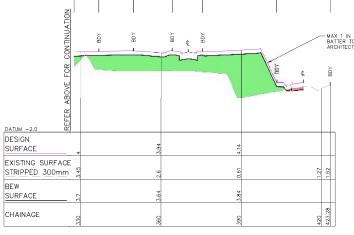


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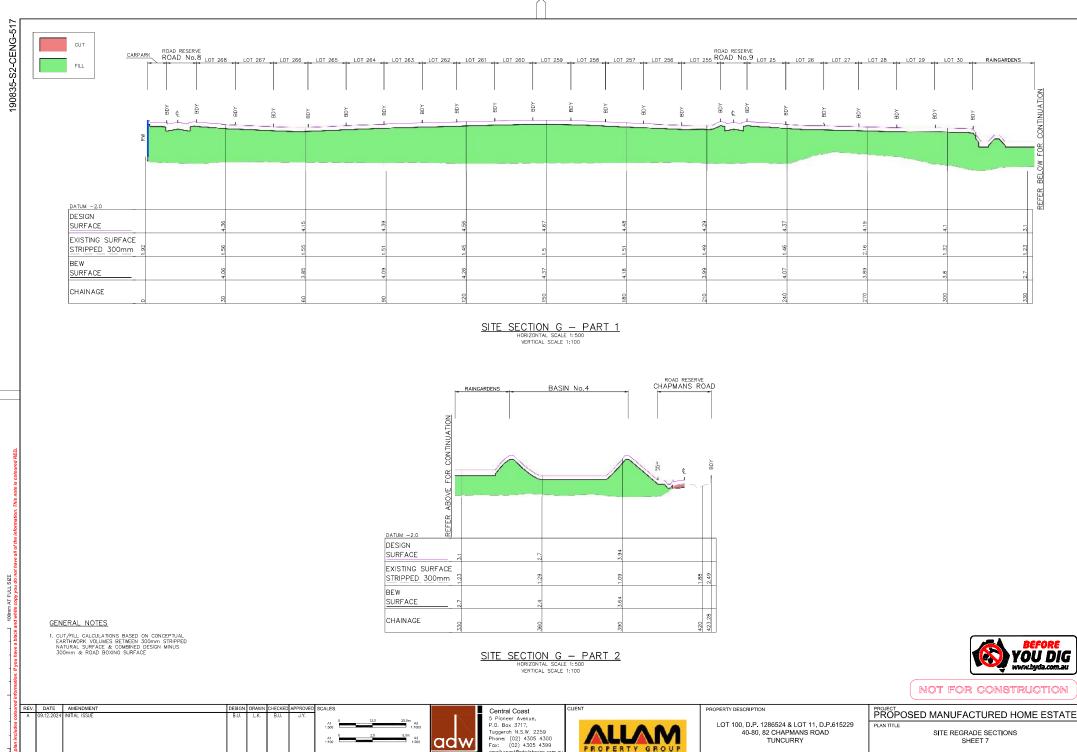
GENERAL NOTES

 CUT/FILL CALCULATIONS BASED ON CONCEPTUAL EARTHWORK VOLUMES BETWEEN 300mm STRIPPED NATURAL SURFACE & COMBINED DESIGN MINUS 300mm & ROAD BOXING SURFACE



SITE SECTION F - PART 2

VERTICAL SCALE 1:100



email: coast@adwjohnson.con

ohnson www.adwjohnson.com.au ABN 62 129 445 398

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ADW Johnson

GDA2020 M.G.A. ZONE 56 A.H.D.

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S2 - CENG - 517

REV.



MINIMISE SEDIMENTATION

INTERVALS.

5.

7.

8.

16.

20.

MATERIAL.

OCCURS.

- DISTURBED AREAS TO BE KEPT TO A MINIMUM. NO MORE THAN 2.5HA OF THE SITE SHALL BE EXPOSED TO EROSION AT ANY ONE TIME.

- CONSERVE ALL TOPSOIL, STOCKPILE AND PROTECT FOR REUSE ON

MAINTAIN ALL EROSION AND SEDIMENT CONTROL MEASURES UNTIL COMPLETE REHABILITATION IS ACHIEVED.

CONSTRUCT STABILISED EARTH BERMS TO DIRECT CLEAN RUNOFF FROM ENTERING THE DISTURBED SITE.

10. WATER QUALITY BASIN TO BE USED AS A TEMPORARY SEDIMENT BASIN DURING CONSTRUCTION

CONSTRUCT STABILISED DIVERSION BANKS TO COLLECT RUNOFF FROM DISTURBED AREAS AND DIRECT IT TO A SEDIMENT BASIN.

PLACE SEDIMENT INLET TRAPS AROUND ALL PITS WITHIN AND DOWNSTREAM OF THE DEVELOPMENT.

13. PLACE GRAVEL BAG GROYNES IN GUTTERS AT 20 - 25m

14. PLACE STRAWBALES IN SWALES AT 40 - 50m INTERVALS.

RUNOFF ENTERING DRAINAGE SYSTEM.

PROTECTED AS SOON AS PRACTICAL

WITH THE APPROVED CMP.

15. PLACE STRAWBALES ACROSS OVERLAND FLOW PATH PRIOR TO THE

STOCKPILES OF MATERIAL TO BE PLACED AWAY FROM DRAINAGE FLOW PATHS AND HEAVILY TRAFFICABLE AREAS AND TO BE SURROUNDED BY SILT FENCING AT ALL TIMES.

17. CONSTRUCT AN ALL WEATHER CONSTRUCTION ACCESS TO THE SITE.

18. ALL DISTURBED AREAS ARE TO BE REVEGETATED OR OTHERWISE

ERECT AND MAINTAIN SILT FENCES AT THE DOWNSLOPE SIDE OF DISTURBED AREA DURING CONSTRUCTION.

22. TREES TO BE RETAINED WITHIN THE CONSTRUCTION AREAS ARE TO BE PROTECTED BY TREE PROTECTION FENCING IN ACCORDANCE

23. ESTABLISH A RESTRICTION BOUNDARY AROUND PROTECTED PLANT WITH PARAWEB FENCING. TEMPORARILY RELOCATE FENCE TO ALLOW CONSTRUCTION OF REQUIRED WORKS AND RE-ESTABLISH PROTECTION ZONE AFTER WORKS COMPLETES.

24. THE SEDIMENT BASINS WILL REQUIRE MAINTENANCE THROUGHOUT THE CONSTRUCTION PROCESS. ADDITIONALLY, THE SEDIMENT BASINS WILL REQUIRE FLOCCULATION IN ACCORDANCE WITH APPENDIX E OF THE 'BLUE BOOK'. THE CONTRACTOR IS TO ABIDE BY APPENDIX E

25. THE CONTRACTOR IS TO INSPECT, CLEAN AND REPAIR ALL EROSION AND SEDMENT CONTROL MEASURES THROUGHOUT THE CONTROL AND SEDMENT CONTROL MEASURES THROUGHOUT THE CONTROL AND SEDMENT CONTROL MEASURES ARE INSPECTED DULY AGN AND FOR THE STORM EVENTS BY THE CONTRACTOR. THE CONTRACTOR IS TO CLEAN AND REPAIR EROSION AND SEDMENT CONTROL MEASURES TO ENSURE THEY ARE ABLE TO COMPLETE THE REQUIRED TASK AS PERFIRST INSTALLATION.

EROSION CONTROL SHOWN IS INDICATIVE ONLY AND SHALL BE A GUIDE FOR THE CONTRACTOR TO PREPARE THEIR OWN EROSION CONTROL DECUMENTATION PRIOR TO CONSTRUCTION.

OF THE 'BLUE BOOK' TO ENSURE ADEQUATE FLOCCULATION

AREAS OUTSIDE THE BOUNDARIES OF THE PROPOSED DEVELOPMENT WILL BE FENCED WITH NO GO FENCING TO KEEP THE AREAS FREE FROM DISTURBANCE OF MACHINERY, PARKED VEHICLES AND WASTE

- 4. KEEP CLEAN WATER SEPARATE FROM DIRTY WATER.

PROTECT ALL DISTURBED AREAS FROM EROSION.



3. CONTROL CLEAN WATER FROM ABOVE THE SITE, THROUGH THE SITE.

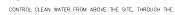








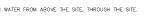














SANDBAGS OVERLAP ONTO KERB -

CONSTRUCTION SITE

RUNDEE DIRECTED TO

DUNOF

GAP BETWEEN BAGS

ACT AS SPILLWAY

TIMBER SLEEPERS OR METAL RAILS 100mm HIGH 200 CTS

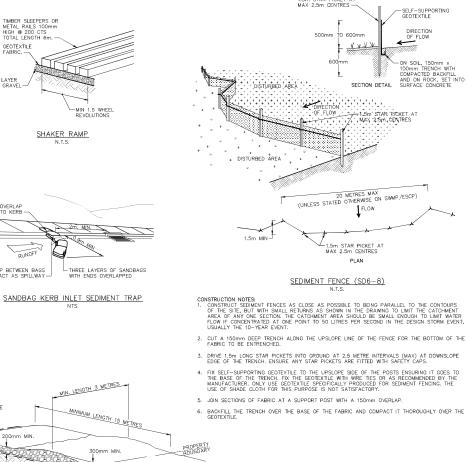
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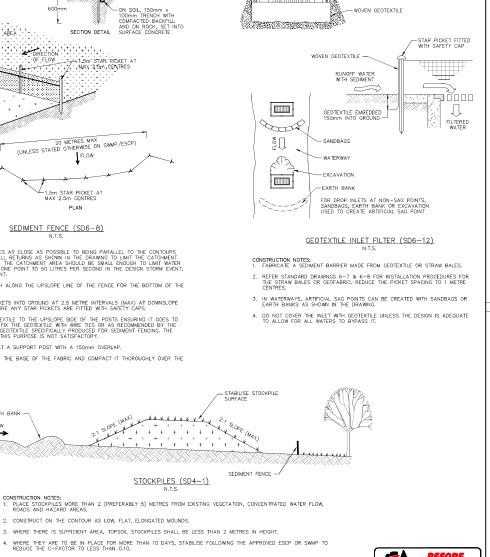
NTS

TOTAL LENGTH 6n

GEOTEXTILE

FARRIC





5. CONSTRUCT EARTH BANKS (STANDARD DRAWING 5-5) ON THE UPSLOPE SIDE TO DIVERT WATER AROUND STOCKPILES AND SEDIMENT FENCES (STANDARD DRAWING 6-8) 1 TO 2 METRES DOWNSLOPE.



NOT FOR CONSTRUCTION

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12			johnson.com.au	SURVEYED DATUM	PROJECT No. PREFIX DISCIPLINE NUMBER REV.
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- 26

GEOTEXTILE FABRIC DESIGNED TO PREVENT INTERMINING OF SUBGRADE AND BASE MATERIALS AND TO MAINTAIN GOOD PROPERTIES OF THE SUB-BASE LAYERS. GEOFABRIC MAY BE A WOVEN OR NEEDLE-PUNCHED PRODUCT WITH A MINIMUM GER BURST STRENCTH (AS3706.4-90) OF 2500N.

200mm MIN

R. C. Land

STABILISED SITE ACCESS (SD6-14) N.T.S.

- CONSTRUCTION NOTES: 1. STRIP THE TOPSOIL, LEVEL THE SITE AND COMPACT THE SUBGRADE.
- 2. COVER THE AREA WITH NEEDLE-PUNCHED GEOTEXTILE.
- 3. CONSTRUCT A 200mm THICK PAD OVER THE GEOTEXTILE USING ROAD BASE OR 30mm AGGREGATE
- ENSURE THE STRUCTURE IS AT LEAST 15 METRES LONG OR TO BUILDING ALIGNMENT AND AT LEAST 3 METRES WIDE.
- WHERE A SEDIMENT FENCE JOINS ONTO THE STABILISED ACCESS, CONSTRUCT A HUMP IN THE STABILISED ACCESS TO DIVERT WATER TO THE SEDIMENT FENCE.

EXISTING ROADWA

1.5m STAR RICKET AT

STAR

PICKETS

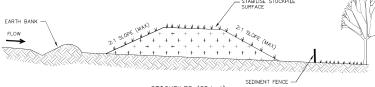
1m MAX

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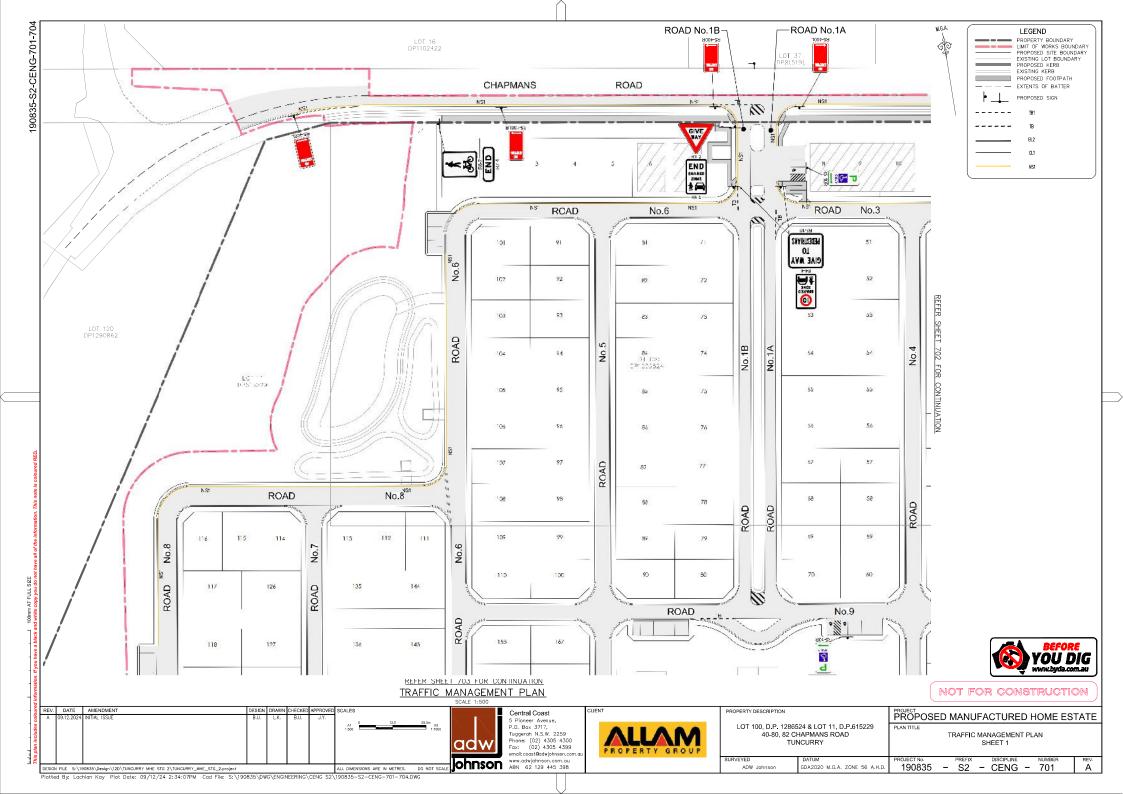
-DROP INLET WITH GRATE

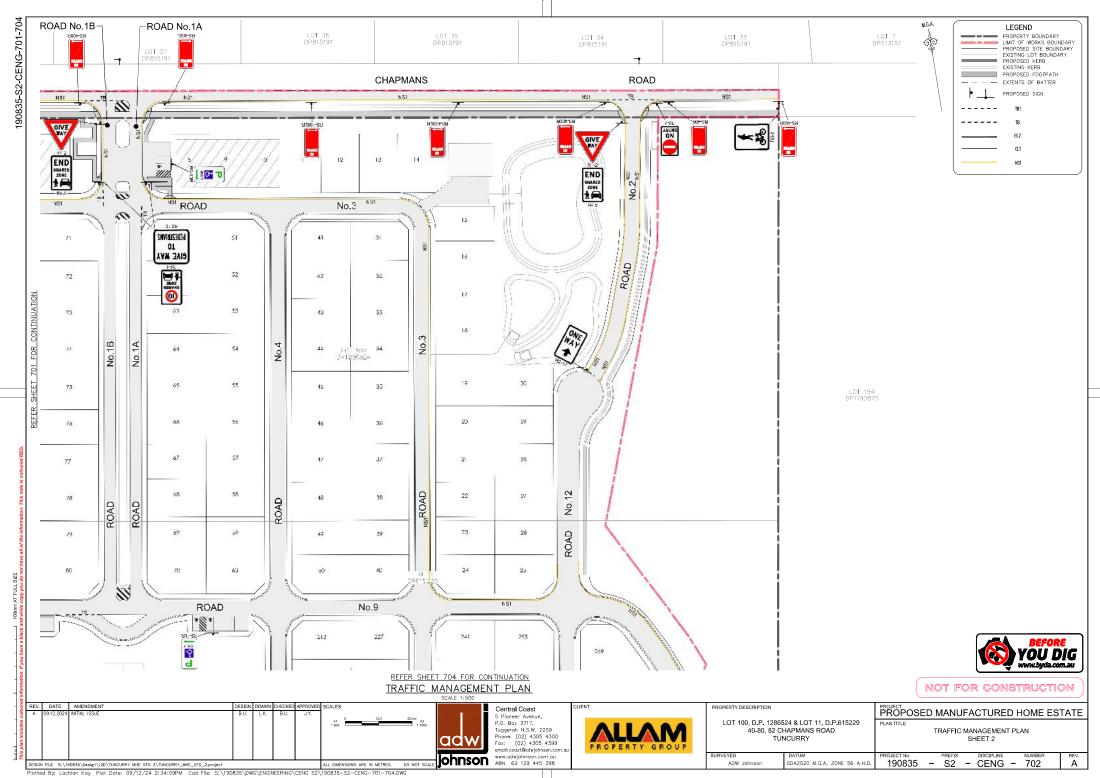
WIRE OR STEEL MESH (14 GAUGE x

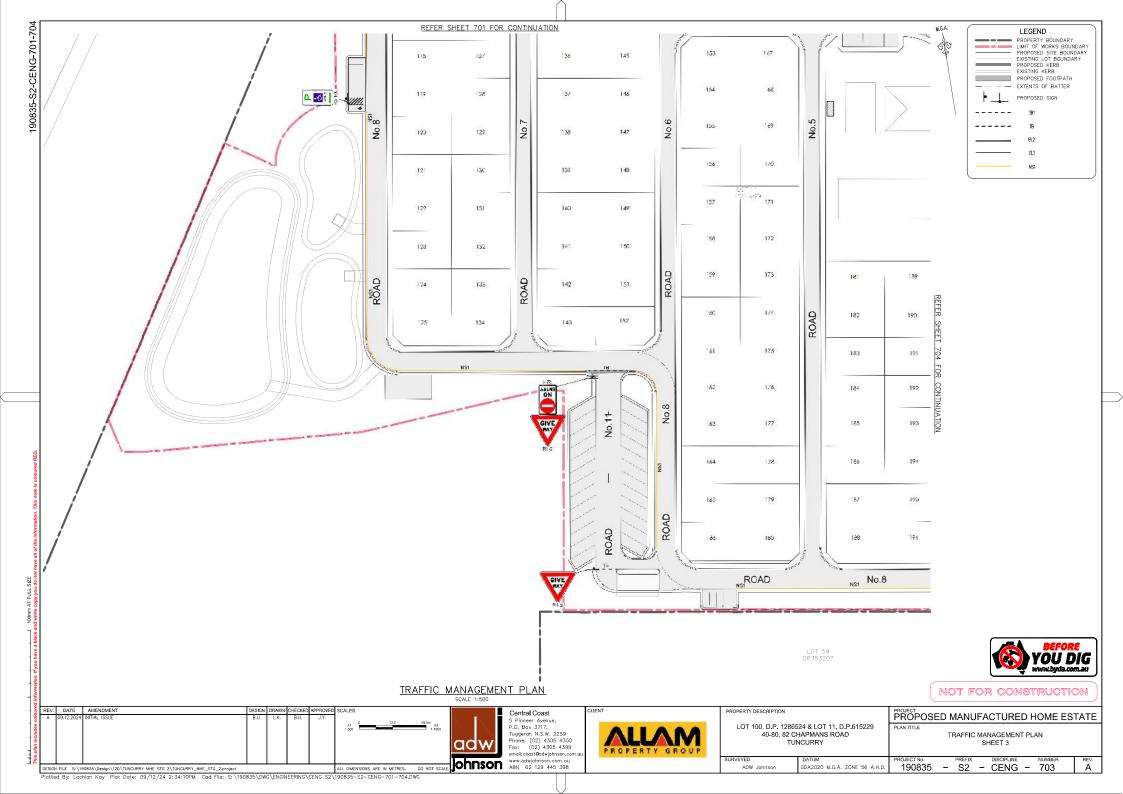
150mm OPENINGS) WHERE GEOTEXTILE IS NOT SELF-SUPPORTING



CONSTRUCTION NOTES: 1. PLACE STOCKPILES MORE THAN 2 (PREFERABLY 5) METRES FROM EXISTING VEGETATION, CONCENTRATED WATER FLOW. ROADS AND HAZARD AREAS.



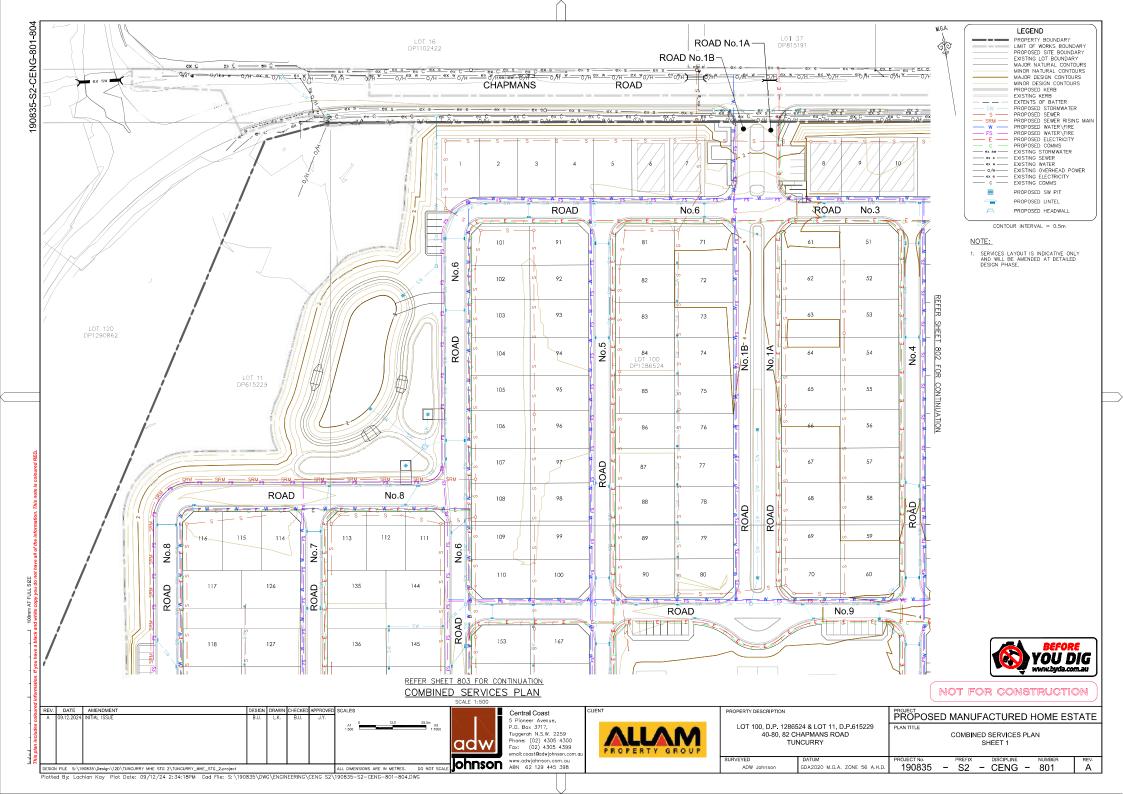


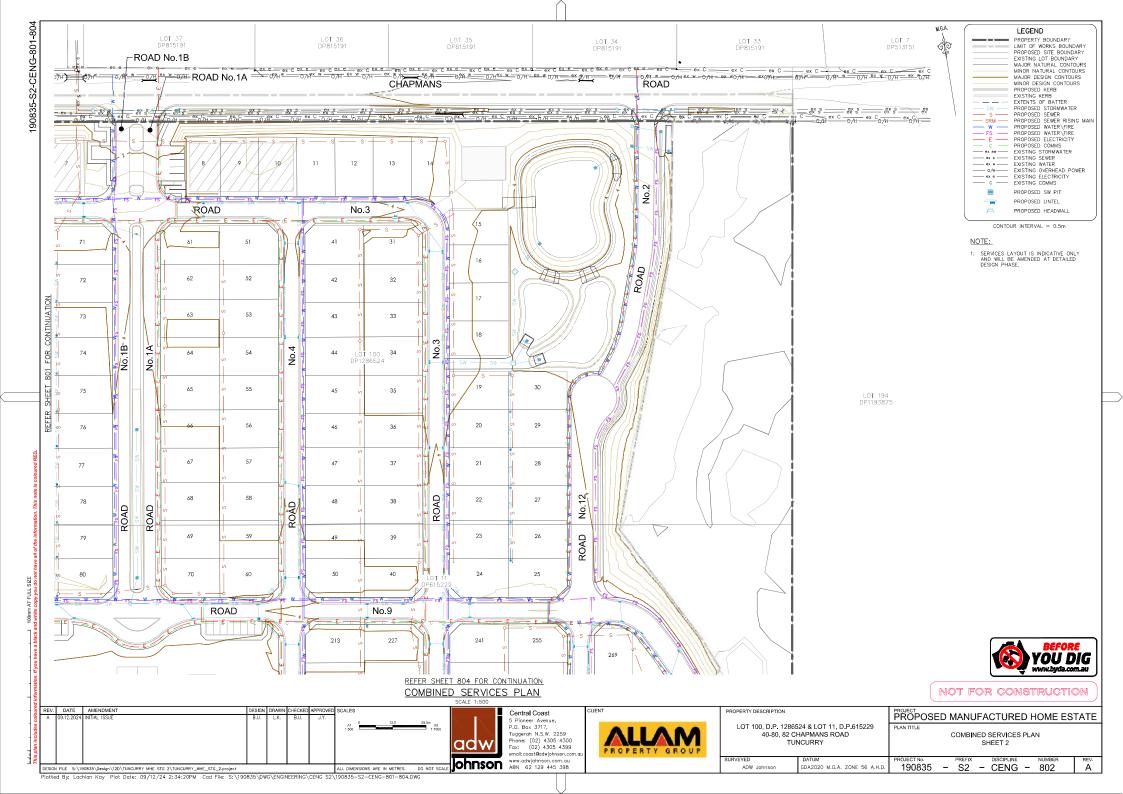


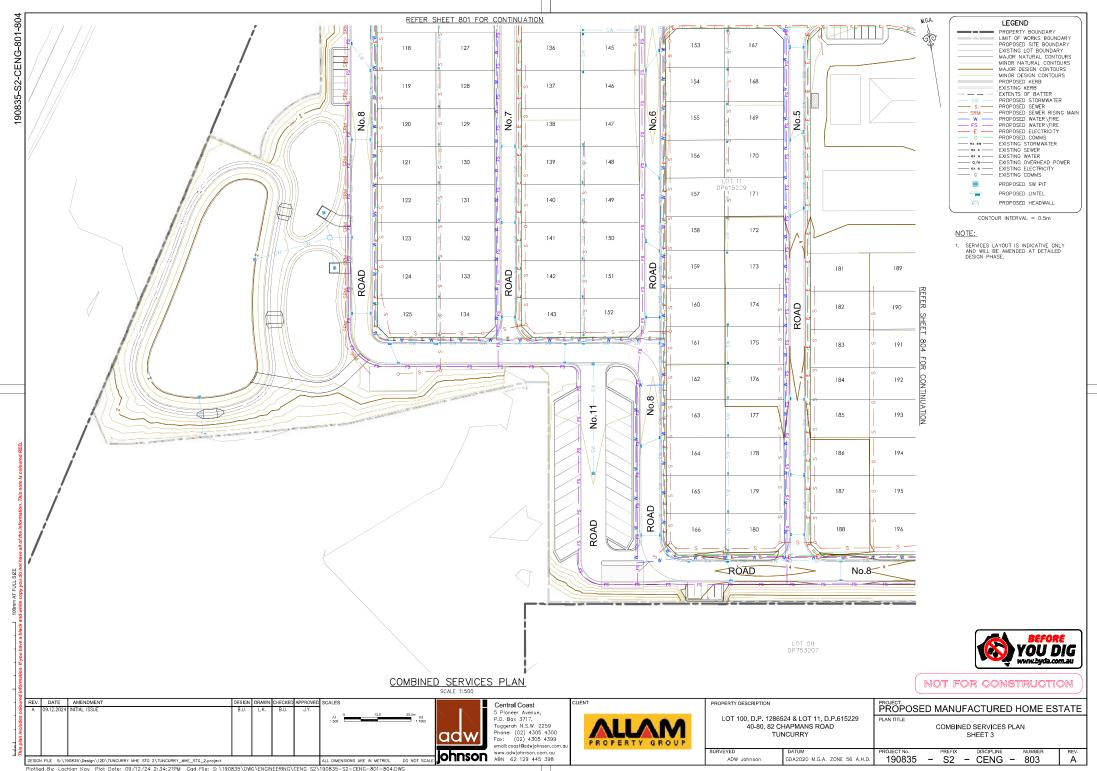


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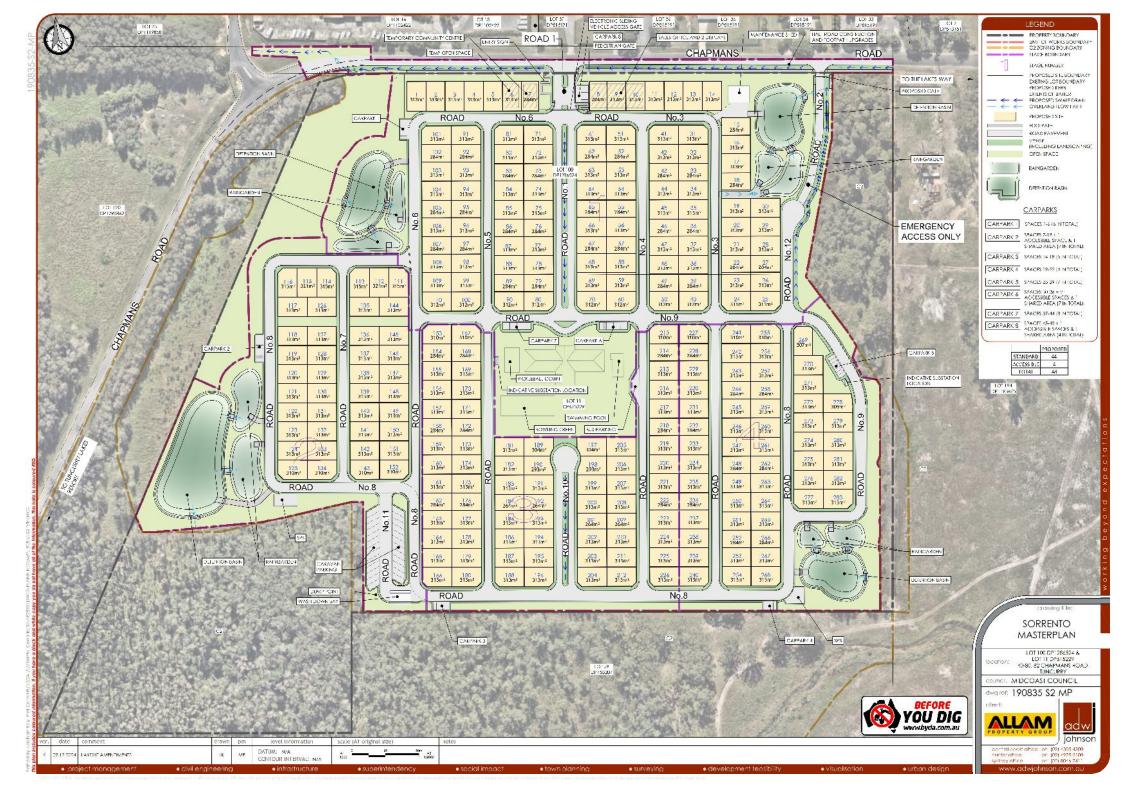








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Appendix B

About this Report

Introduction

These notes have been provided to amplify Douglas' report in regard to classification methods, field procedures and the comments section. Not all are necessarily relevant to all reports.

Douglas' reports are based on information gained from limited subsurface excavations and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

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This report is the property of Douglas Partners Pty Ltd. The report may only be used for the purpose for which it was commissioned and in accordance with the Engagement Terms for the commission supplied at the time of proposal. Unauthorised use of this report in any form whatsoever is prohibited.

Borehole and Test Pit Logs

The borehole and test pit logs presented in this report are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling or excavation. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable or possible to justify on economic grounds. In any case the boreholes and test pits represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes or pits, the frequency of sampling, and the possibility of other than 'straight line' variations between the test locations.

Groundwater

Where groundwater levels are measured in boreholes there are several potential problems, namely:

- In low permeability soils groundwater may enter the hole very slowly or perhaps not at all during the time the hole is left open;
- A localised, perched water table may lead to an erroneous indication of the true water table;
- Water table levels will vary from time to time with seasons or recent weather

changes. They may not be the same at the time of construction as are indicated in the report; and

• The use of water or mud as a drilling fluid will mask any groundwater inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water measurements are to be made.

More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from a perched water table.

Reports

The report has been prepared by qualified personnel, is based on the information obtained from field and laboratory testing, and has been undertaken to current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal, the information and interpretation may not be relevant if the design proposal is changed. If this happens, Douglas will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface conditions, discussion of geotechnical and environmental aspects, and recommendations or suggestions for design and construction. However, Douglas cannot always anticipate or assume responsibility for:

- Unexpected variations in ground conditions. The potential for this will depend partly on borehole or pit spacing and sampling frequency;
- Changes in policy or interpretations of policy by statutory authorities; or
- The actions of contractors responding to commercial pressures.

If these occur, Douglas will be pleased to assist with investigations or advice to resolve the matter.



About this Report

Site Anomalies

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, Douglas requests that it be immediately notified. Most problems are much more readily resolved when conditions are exposed rather than at some later stage, well after the event.

Information for Contractual Purposes

Where information obtained from this report is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section is not relevant to the contractual situation, it may be appropriate to prepare a specially edited document. Douglas would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

Site Inspection

The company will always be pleased to provide engineering inspection services for geotechnical and environmental aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

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Appendix C

RGS Bore Logs (MW1 to MW4)

Douglas Bore Logs (101 to 105) (219536.00 and 219536.01)

Slug Test Analysis Report Sheets (MW1, MW3, MW4 and 101 to 104)

Double Ring Infiltrometer Test Sheets (201 to 212)

Figure 1 – Groundwater Level vs Rainfall (MW1, MW3, MW4 and 105)

Figure 2 – Groundwater Level vs Rainfall (101 to 104)

				E	INGI	NEE	RING LOG - BOREHOLE			E	ORE	HOLE	E NO: MW1
		REGION/ GEOTEC		~			Allam Property Group			P	AGE	E:	1 of 1
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				тт	EST L	OCAT	ION: See Figure 3			D	ATE	:	7/10/22
		YPE:		oota Exca			EASTING:			SURF		RL:	
во				: 100 n	nm	IN	CLINATION: 90° NORTHING:		[DATU		d Test	AHD
	Drii	ling and Sai	mpling			z	Material description and profile information				Fiel		
METHOD	WATER	SAMPLES	RL (Not measured	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticit characteristics,colour,minor componen		MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T				-		SC	TOPSOIL: Clayey SAND, fine to medium <u>c</u> dark grey/black, clay, low plasticity, some r		M				TOPSOIL
	I			0.2		SP	SAND: Fine to medium grained, grey/pale		-				AEOLIAN — — — — — — — — — — — — — — — — — — —
				0.8									
LEC Wat							2.00m Hole Terminated at 2.00 m						
LEG	END:		L	Notes, Sar	mples an	l Id Tests	2,0011	Consiste		1		CS (kPa	
<u>Wat</u> ⊻	Wat (Dat	ter Level te and time s ter Inflow ter Outflow	hown)	U₅₀ CBR E ASS B	Bulk s Enviro Acid S	ample f	ter tube sample or CBR testing I sample Soil Sample	S S F F St S VSt V H F	/ery Soft Soft Stiff /ery Stiff lard Friable		25 50 10 20	25 5 - 50 0 - 100 00 - 200 00 - 400 400	P
	G tra D	radational or ansitional str efinitive or di rata change	ata	Field Test PID DCP(x-y) HP	Photo Dynar	nic pene	on detector reading (ppm) etrometer test (test depth interval shown) meter test (UCS kPa)	Density	V L MC D VD	L N N D	ery Lo bose lediun ense ery De	n Dense	Density Index <15% Density Index 15 - 35% Density Index 35 - 65% Density Index 65 - 85% Density Index 85 - 100%

		REGION	AI				RING LOG - BORE				B	BORE	EHOLI	E NO:	MW2
		GEOTEC	HNICA	L	LIENT		Allam Property Gro	qu				AGE			1 of 1
_		SOLUTIO	DNS		ROJE						J	OBI	NO:		RGS03137.1
							•	oad, Tuncurry					GED E	BY:	APH
				Т	EST L	OCAT	ION: See Figure 3				D	DATE			7/10/22
		TYPE: OLE DIAN		ota Exca		INI	CLINATION: 90°	EASTING: NORTHING:			SURF. DATU		RL:	AH	D
БО		ling and Sa		. 100 11		IN	Material description and				JATU	-	d Test		D
	Dill					z						Fiel			
METHOD	WATER	SAMPLES	RL (Not measured)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION characteristics,colo	l: Soil type, plasticity ur,minor components		MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Stru	cture and additional observations
AD/T				-		SC	TOPSOIL: Clayey SAN dark grey/black, clay, lo			М				TOPSO	IL.
				0.2		SC	0.20m Clayey SAND: Fine to brown, clay, low plastici		 vn/dark					LIGHTL	Y INDURATED SAND
	4 7/10/2022			0.8 		SP	1.00m	grained, grey/pale g						AEOLIA	<u></u>
						5	brown	granoa, groyipalo g							
				-			2.00m Hole Terminated at 2.0) m							
	END:			Notes, Sar	mples an	d Tests			Consiste	ency Very Soft			CS (kPa 25	a) <u>Mois</u> D	ture Condition Dry
Wat		ter Level					ter tube sample		S	Soft		25	5 - 50	м	Moist
	(Dat	te and time s	shown)	CBR E			or CBR testing I sample			Firm Stiff) - 100)0 - 200) W Wp	Wet Plastic Limit
		ter Inflow ter Outflow		ASS B	Acid S	Sulfate S	Soil Sample		VSt	Very Stiff Hard		20	00 - 400 400	P	Liquid Limit
Stra	ta Cha					Sample			Fb	Friable					
	–– G tra –– De	radational or ansitional str efinitive or di rata change	ata	Field Test PID DCP(x-y) HP	Photo Dynar	nic pene	on detector reading (ppm) etrometer test (test depth interval meter test (UCS kPa)	shown)	<u>Density</u>	V L D VD	La D M D	ery Lo oose lediun ense ery De	n Dense	Dens e Dens Dens	iity Index <15% iity Index 15 - 35% iity Index 35 - 65% iity Index 65 - 85% iity Index 85 - 100%

		REGION					RING LOG - BOREHOLE					EHOLE	
		GEOTEC	HNICA	L	ROJEC		Allam Property Group ME: Proposed MHE						1 of 1
		SULUTIU	IN S		ITE LO		·				OBI	NO: GED E	RGS03137.1 3Y: APH
							ION: See Figure 3) ATE		7/10/22
DR		TYPE:	6T Kuł	oota Exc			EASTING:			SURF			
		OLE DIAN				IN	CLINATION: 90° NORTHING:			DATU			AHD
	Dril	ling and Sar	mpling				Material description and profile information				Fie	d Test	
METHOD	WATER	SAMPLES	RL (Not measured)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity characteristics,colour,minor component:		MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T				0.2		CL SC	TOPSOIL: Silty CLAY, low plasticity, dark grey/black, some sand, fine to medium grair roots 0.25m Clayey SAND: Fine to coarse grained, pale grey/pale brown, clay, low plasticity, some n		M				TOPSOIL
				0.4									
	I 7/10/2022			0.8_ - - 1.0_ -									
				- 1.2_ - -									
				1.4 									
	END:			-		d Toot	2.00m Hole Terminated at 2.00 m	Consist				CS (kPa) Moisture Condition
<u>Wat</u> ▼	er Wa (Da Wa Wa	ter Level te and time s ter Inflow ter Outflow	hown)	U₅₀ CBR E ASS B	50mm Bulk s Enviro	Diame ample f nmenta fulfate \$	e ter tube sample for CBR testing al sample Soil Sample	S F St VSt H	Very Soft Soft Firm Stiff Very Stiff Hard		<: 25 50 10 20	<u>CS (kPa</u> 25 5 - 50 0 - 100 00 - 200 00 - 400 400	D Dry M Moist W Wet W _p Plastic Limit
<u>Stra</u> — –	G tra D	anges radational or ansitional stra efinitive or dia rata change	ata	Field Test PID DCP(x-y) HP	Photoi Dynan	nic pen	on detector reading (ppm) etrometer test (test depth interval shown) ometer test (UCS kPa)	Fb Density	Friable V L M D V	La D M D	ery Lo oose lediur ense ery D	n Dense	Density Index <15% Density Index 15 - 35% Density Index 35 - 65% Density Index 65 - 85% Density Index 85 - 100%

	-	REGION	M				RING LOG - BORE				E	BORE	EHOLI	e no:	MW4
		GEOTEC	HNICAI	L			Allam Property Gr	oup				AGE			1 of 1
-		SOLUTIO	INS		ROJE							OBI			RGS03137.1
							•	Road, Tuncurry					GED E	BY:	APH
				Т	EST L	UCAT	ON: See Figure 3				C	OATE	:		7/10/22
				ota Exc				EASTING:			SURF		RL:		2
ВС		OLE DIAN		100 n	nm	IN	CLINATION: 90°	NORTHING:			DATU	-		AH)
	Dril	ling and Sar	mpling T			-7	Material description and	I profile information				Fiel	d Test	-	
METHOD	WATER	SAMPLES	RL (Not measured)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTIO characteristics,col	N: Soil type, plasticity our,minor components		MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Struc	ture and additional observations
AD/T	7/10/2022			0.2		SC	FILL: Clayey SAND, f grey/black, clay, low p		d, dark	М				FILL/TO	PSOIL
	21/2			0.6		SC	0.50m TOPSOIL: Clayey SA dark grey/black, some		 rained,					TOPSOI	<u>.</u>
LEC Wat				0.8 1.0 		SC	Clayey SAND: Fine to brown, clay, low plasti		 wn/pale	W				LIGHTLY	Y INDURATED SAND
						SC	1.20m Clayey SAND: Fine to grey/pale brown, clay,		2					ĀLLUVI/	<u>al soil</u> — — — — — — — — — — — — — — — — — — —
				-			2.00m Hole Terminated at 2.	00 m							
LEG	SEND:	I	<u>1</u>	Notes, Sar	mples an	d Tests	2.0011		Consist			_	CS (kPa		ure Condition
	Wa (Da Wa	ter Level te and time s ter Inflow ter Outflow	hown)	U₅₀ CBR E ASS B	Bulk s Enviro Acid S	ample f	ter tube sample or CBR testing I sample toil Sample		VS S F St VSt H	Very Soft Soft Firm Stiff Very Stiff Hard		25 50 10 20	25 5 - 50 0 - 100 00 - 200 00 - 400 400	P	Dry Moist Wet Plastic Limit Liquid Limit
<u>Stra</u>	tra D	anges radational or ansitional stra efinitive or dia rata change	ata	Field Test PID DCP(x-y) HP	Photo Dynar	nic pene	n detector reading (ppm) trometer test (test depth interva meter test (UCS kPa)	al shown)	Fb Density	Friable V L D VD	L N D	ery Lo oose lediun ense ery De	n Dense	Densi e Densi Densi	ty Index <15% ty Index 15 - 35% ty Index 35 - 65% ty Index 65 - 85% ty Index 85 - 100%

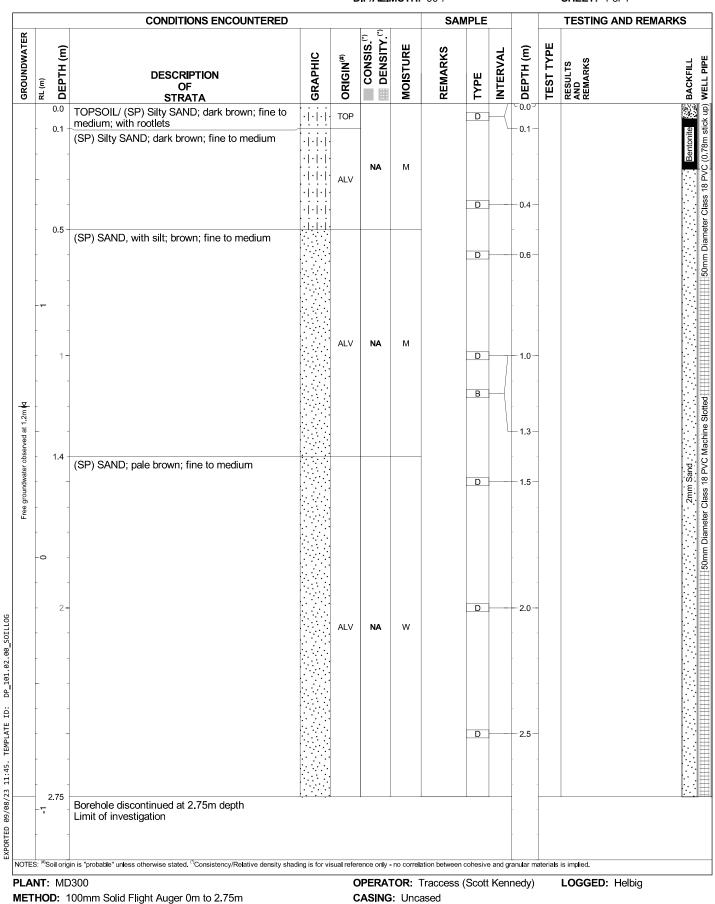
CLIENT:

Allam Property Group

LOCATION: 82 Chapmans Road, Tuncurry

PROJECT: Proposed Manufactured Home Estate

SURFACE LEVEL: 1.8 AHD COORDINATE E:451334.1 N: 6441896.8 DATUM/GRID: GDA94 DIP/AZIMUTH: 90°/--- LOCATION ID: 101 PROJECT No: 219536.01 DATE: 29/06/23 SHEET: 1 of 1



Refer to explanatory notes for symbol and abbreviation definitions

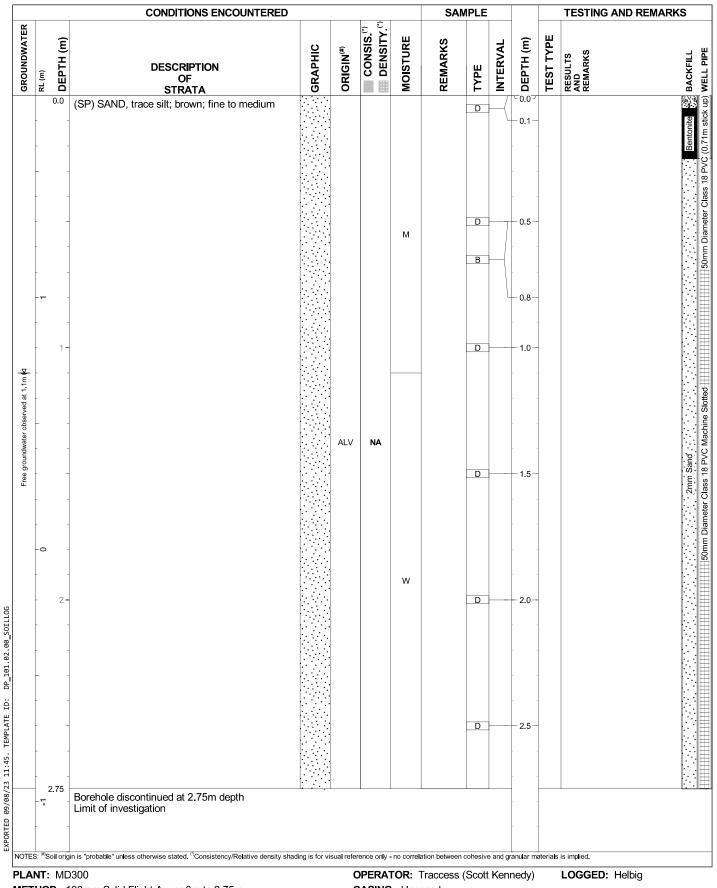
REMARKS:

CLIENT: Allam Property Group PROJECT: Proposed Manufactured Home Estate LOCATION: 82 Chapmans Road, Tuncurry

BOREHOLE LOG

SURFACE LEVEL: 1.8 AHD COORDINATE E:451166.6 N: 6441792.5 DATUM/GRID: GDA94 DIP/AZIMUTH: 90°/---

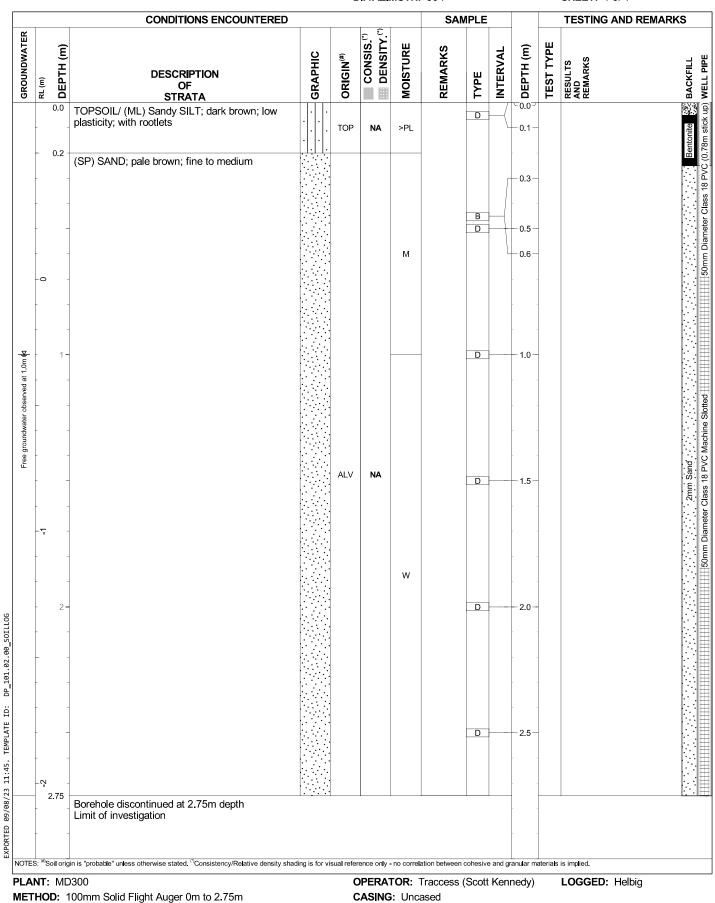
LOCATION ID: 102 PROJECT No: 219536.01 DATE: 29/06/23 SHEET: 1 of 1



METHOD: 100mm Solid Flight Auger 0m to 2.75m **REMARKS**:

CASING: Uncased

SURFACE LEVEL: .7 AHD COORDINATE E:450939.5 N: 6442075.1 DATUM/GRID: GDA94 DIP/AZIMUTH: 90°/--- LOCATION ID: 103 PROJECT No: 219536.01 DATE: 29/06/23 SHEET: 1 of 1



REMARKS:

CLIENT:

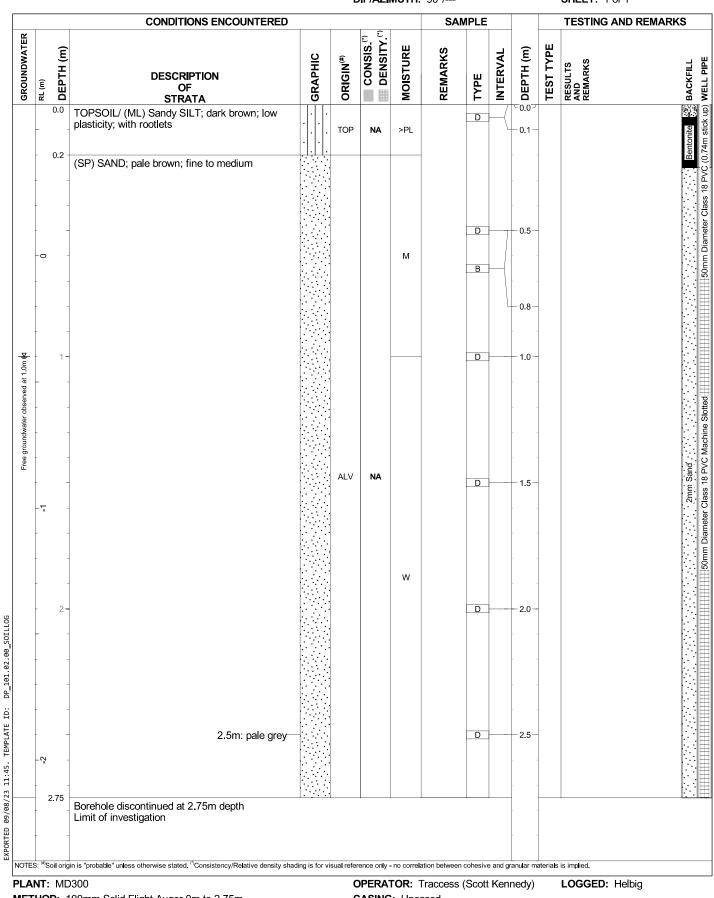
Allam Property Group

LOCATION: 82 Chapmans Road, Tuncurry

PROJECT: Proposed Manufactured Home Estate

SURFACE LEVEL: .6 AHD COORDINATE E:450849.2 N: 6441931.7 DATUM/GRID: GDA94 DIP/AZIMUTH: 90°/---

LOCATION ID: 104 PROJECT No: 219536.01 DATE: 29/06/23 SHEET: 1 of 1



METHOD: 100mm Solid Flight Auger 0m to 2.75m **REMARKS**:

CLIENT:

Allam Property Group

LOCATION: 82 Chapmans Road, Tuncurry

PROJECT: Proposed Manufactured Home Estate

CASING: Uncased

CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 1.8 AHD COORDINATE: E:451382.0, N:6442118.8 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 105 **DATE:** 13/08/24 SHEET: 1 of 1

		CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m) DEPTH (m)	DESCRIPTION OF STRATA	CRAPHIC	ORIGIN ^(#)	CONSIS. ^(*) DENSITY. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS

CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 2.1 AHD COORDINATE: E:451309.4, N:6442111.6 **PROJECT No:** 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 201 **DATE:** 30/11/23 SHEET: 1 of 1

		CONDITIONS ENCOUNTERED			_ £		SAN	IPLE				TESTING AND REMARKS
	BL (m) DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
	-	FILL / Silty SAND (SP): dark brown; fine to medium; trace shell, gravel, plastic.		FILL	ND	М				-	-	
	0.25	FILL / SAND (SP), trace gravel: brown; fine to medium; fine, sub-angular gravel.							Г	- - - 0.50 -	-	
-				FILL	ND	D		В		-	-	
-	1.00	Silty SAND (SP): dark grey; fine to medium; trace sulfur odour.								- 1.00 - -	-	
-					ND	м				-	-	
_	-	Borehole discontinued at 1.50m depth. Limit of investigation.										
										-	*	
)TES:	#Soil ori	gin is "probable" unless otherwise stated. ⁽¹⁾ Consistency/Relative density	y shading	is for visua	Il referenc	e only - no	correlation	betwee	n cohes	ive and	granula	ar materials is implied.
_AN ETH	IT: Ha IOD:	and Auger Hand auger to 1.5m • DRI @ 0.5m. Coordinates and elevations me		C	PERA	TOR:	Douglas					LOGGED: Lambert CASINC: Nil

CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 1.5 AHD COORDINATE: E:451256.7, N:6441974.4 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 202 **DATE:** 30/11/23 SHEET: 1 of 1

			1	1	£		SAI	IPLE		4		TESTING AND REMARKS
GROUNDWATER	RL (m) DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
		Silty SAND (SP): dark grey; fine to medium; with rootlets.			ND	м			-			
-	0.15	SAND (SP), with silt: grey; fine to medium.		_						-	-	
										- 0.20 -	-	
-								В		-	-	
-	-	From 0.40m: brown								- 0.40 -	-	
-					ND	М					-	
-		-								-	-	
-										-	-	
23										-	-	
30/11/23						w				-		
erved at c	1 -	Borehole discontinued at 1.00m depth. Limit of investigation.										
Free groundwater observed at 0.9m										-	-	
ee ground										-	-	
Ī,										-		
-	o									-		
-												
+										Ī		
-												
ļ										ľ	ĺ	
+		1								+		
LAI	NT: H	igin is "probable" unless otherwise stated. "Consistency/Relative densit and Auger Hand auger to 1.0m	/ shading				o correlation Douglas					ar materials is implied. LOGGED: Lambert CASING: Njl

CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 1.7 AHD COORDINATE: E:451237.6, N:6441826.0 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 203 **DATE:** 30/11/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAM	1PLE				TESTING AND REMARKS
GROUNDWATER	RL (m) DEDTU ()	UEPIH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
		-	Silty SAND (SP): grey; fine to medium; with		-			-		-			
			rootlets.			ND	м						
İ		1											
	0.	15 -	SAND (SP): pale grey; fine to medium.		-								
Ī		1								[- 0.20 -		
		1											
									В				
		1				ND	м			$ \rangle$			
										$ \rangle$	- 0.50 -		
]									- 0.50 -		
ļ													
-	0.5	70											
			Silty SAND (SP): dark grey; fine to medium.										
30/1/23						ND	w						
2 2 2													
Free groundwater observed at 0.8m		+	Borehole discontinued at 0.90m depth.										
rved			Limit of investigation.										
obse	٦	1 -									- 1 -		
vater													
, puno		1											
ee du													
Ê [1											
		1											
		1											
ļ											L _		
ł											-		
با	þ												
ł		+										-	
ł													
IOTES	: ^供 Soil	orig	in is "probable" unless otherwise stated. "Consistency/Relative densit	y shading	is for visu	ual referenc	e only - n	o correlation b	petwee	n cohes	ive and	granula	ar materials is implied.
LA	NT:	Ha	and Auger		(OPERA	TOR:	Douglas	Part	ners			LOGGED: Lambert
			Hand auger to 0.9m										CASING: Nil
EM.	ARK	S:	DRI @ 0.2m. Coordinates and elevations me typical accuracy of ±0.1m.	easure	d by d	IGPS w	ith						

CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 1.1 AHD COORDINATE: E:451081.7, N:6441812.1 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 204 **DATE:** 30/11/23 SHEET: 1 of 1

1		CONDITIONS ENCOUNTERED)		₽		SAN	MPLE				TESTING AND REMARKS
	RL (m) DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
<u>,</u>	_	Silty SAND (SP): dark grey; fine to medium; with rootlets.	×××		ND	м					-	
23.	0.10	SAND (SP): brown; fine to medium; trace rootlets.				м				- 0.10 -	-	
30/1/23					ND	w		B		50 -	-	
-		Borehole discontinued at 0.60m depth. Limit of investigation.									-	
-											-	
-	1 -									- 1 -	-	
-											-	
											-	
-	-										-	
TES	: [#] Soil ori	gin is "probable" unless otherwise stated. "Consistency/Relative densi	ty shading i	s for visu	al referenc	e only - no	ocorrelation	betweer	n cohes	ive and	granula	ar materials is implied.
		and Auger Hand auger to 0.6m		C	OPERA	TOR:	Douglas	s Part	ners			LOGGED: Lambert CASING: Nil



CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 1.0 AHD COORDINATE: E:451083.7, N:6441909.2 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 205 **DATE:** 30/11/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED	-				SAN	1PLE				TESTING AND REMARKS
GROUNDWATER	RL (m) DEDTH (m)		DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
			Silty SAND (SP): dark grey; fine to medium; with rootlets, roots.	× × × ×		ND	м					-	
	0.1	-	SAND (SP): brown; fine to medium.	×		ND	М					-	
	-	-	Borehole discontinued at 0.50m depth. Limit of investigation.				W						
	-	-										-	
	-	-										-	
	-											-	
	-	-										-	
	ا م	-									- 1 -	-	
	-	-										-	
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	-	-										-	
	-	-										-	
	-												
	-												
	-	+											
	-	-											
NOTE	S: ^{(#} Soil	orig	jin is "probable" unless otherwise stated. ⁽¹⁾ Consistency/Relative densit	y shading is	for visu	al referenc	e only - n	o correlation I	petweer	n cohes	ive and	granula	ar materials is implied.
			and Auger		C	OPERA	TOR:	Douglas	Part	ners			LOGGED: Lambert
			Hand auger to 0.5m DRI @ 0.2m. Coordinates and elevations me	easured	l bv d	GPS w	ith						CASINC: Nil
			typical accuracy of ±0.1m.	Jusureu	, by u	5,5 %							



CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 1.3 AHD COORDINATE: E:451180.9, N:6441968.9 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 206 **DATE:** 30/11/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	IPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
			Silty SAND (SP): dark grey; fine to medium; with rootlets.	××		ND	м						
	-	0.10	SAND (SP): brown; fine to medium.								-	-	
	-	-				ND	М		В		- 0.30 -	-	
m 30/11/23	-						w				- 0.60 -	-	
ed at 0.6	1		Borehole discontinued at 0.70m depth. Limit of investigation.										
observe	-	-	Limit of investigation.								_	-	
ndwater	-										_	_	
Free groundwater observed at 0.6m	-												
Ţ	_	1 -									- 1 -	-	
											-	-	
											-	-	
	_0										-	-	
	-	-									-	-	
	-												
	-												
											-	-	
											-	-	
	[-									_		
	-	-										-	
NOT	C. (#	Soil or	jin is "probable" unless otherwise stated. "Consistency/Relative densi	v choding ⁱⁿ	s for vice	al reforce -	a only in	o correlation '	notwoor	a cobo-	ive and	arapula	r materials is implied
				y snading is									LOGGED: Lambert
			and Auger Hand auger to 0.7m		Ľ	JPERA	IUR:	Douglas	Part	ners			CASING: Nil
			DRI @ 0.3m. Coordinates and elevations mo typical accuracy of ±0.1m.	easured	d by d	GPS w	ith						



CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 2.1 AHD COORDINATE: E:451140.6, N:6442038.5 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 207 **DATE:** 30/11/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED					SAN	MPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
erved			FILL / SAND (SP), trace gravel: brown; fine to medium; fine, sub-angular gravel; trace rootlets.						в				
Ν	-	-			FILL	ND	D				- - 0.50 - -	-	
			From 0.70m: increased gravel content								-	_	
		1 -	Borehole discontinued at 0.90m depth. Refusal on gravel.		<u>100000</u>						- 1 -		
											-	-	
	-										-	-	
	-	-										_	
	-										-	-	
											-	-	
NOTE	S: 件S	50il ori	 gin is "probable" unless otherwise stated. ^{(¶} Consistency/Relative density	/shading i	is for visua	al referenc	e only - n	o correlation	betweer	n cohes	ive and	l granula	r materials is implied.
			and Auger		c	PERA	TOR:	Douglas	s Part	ners			LOGGED: Lambert
			Hand auger to 0.9m										CASING: Nil
REM	1AF	RKS	DRI @ 0.1m. Coordinates and elevations me typical accuracy of ±0.1m.	asurec	l by d(GPS wi	th						



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CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 2.9 AHD COORDINATE: E:451191.8, N:6442104.8 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 208 **DATE:** 30/11/23 SHEET: 1 of 1

		CONDITIONS ENCOUNTERED					SAN	1PLE				TESTING AND REMARKS
30/11/23 No free groundwater observed GROUNDWATER	RL (m) DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
ved		FILL / SAND (SP): pale brown; fine to medium;										
bsen	-	trace shell, rootlets.										
ater o		From 0.10m: no rootlets										
wpu	t			FILL	ND	D				- 0.20 -		
grou						D				- 0.20 -		
o free	Į.							в				
/23 N									\backslash			
30/11	-	Borehole discontinued at 0.40m depth.								- 0.40 -		
		Collapse.										
	-											
	-											
	-											
	-											
	2											
	i 1 -									- 1 -		
	-											
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	-											
	-											
	-											
	-											
	ļ											
	-											
	ŀ											
NOTE	 S: ^{(#} Soil ori	gin is "probable" unless otherwise stated. "Consistency/Relative density	/ shading i	s for visua	al referenc	e only - n	o correlation I	oetweer	n cohes	ive and g	granula	ar materials is implied.
		and Auger		C	PERA	TOR:	Douglas	Part	ners			LOGGED: Lambert
		Hand auger to 0.4m				;+h						CASING: Nil
RCIV	INKA3	 DRI @ 0.2m. Coordinates and elevations me typical accuracy of ±0.1m. 	easured	n by di	942 W	ונו)						



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CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 2.7 AHD COORDINATE: E:451309.5, N:6442104.4 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 209 **DATE:** 01/12/23 SHEET: 1 of 1

			CONDITIONS ENCOUNTERED			-		SAM	1PLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
01/12/23 No free groundwater observed	-	-	FILL / SAND (SP): pale brown; fine to medium; trace shell, gravel.		FILL	ND	D		В				
01/12/23 No	-	-	Borehole discontinued at 0.30m depth. Collapse.								- 0.30 - 	-	
		-								-	· · ·	-	
	-	-									· ·	-	
	- -	-								-	· ·	-	
PLA	NT:	Ha	gin is "probable" unless otherwise stated. "Consistency/Relative densit and Auger Hand auger to 0.3m	y shading i				o correlation t					r materials is implied. LOGGED: Lambert CASING: Nil
		KS:	DRI @ 0.0m. Coordinates and elevations me typical accuracy of ±0.1m.	easure	d by d	GPS w	ith						



Generated with CORE-GS by Geroc - Soil Log - 19/12/2023 9:25:43 AM

CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 2.3 AHD COORDINATE: E:451276.9, N:6442105.6 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 210 **DATE:** 01/12/23 SHEET: 1 of 1

а Н		CONDITIONS ENCOUNTERED			e. 5.		SAN					TESTING AND REMARKS
GROUNDWATER	^{вц (m)} DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)		MOISTURE	REMARKS	түре	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
naviasi	0.10	FILL / Silty SAND (SP): brown; fine to medium; trace rootlets, wood, metal, shell.	× ×	FILL	ND	D						
	0.10	FILL / SAND (SP), with silt: pale brown pale grey; fine to medium; trace shell.								-	-	
-	-	From 0.60m: pale brown, trace silt		FILL	ND	D					-	
	-	Borehole discontinued at 1.20m depth. Limit of investigation.								-	-	
-	-										-	
IOTES:	#Soil orig	gin is "probable" unless otherwise stated. "Consistency/Relative density	[,] shading i	is for visua	il reference	e only - no	o correlation I	petweel	n cohes	ive and	granula	ar materials is implied.
1ETH	HOD:	and Auger Hand auger to 1.2m : DRI @ 0.2m. Coordinates and elevations me		C	PERA	TOR:	Douglas	Part	ners			LOGGED: Lambert CASING: Nil



CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

SURFACE LEVEL: 0.7 AHD COORDINATE: E:450974.5, N:6442099.2 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

LOCATION ID: 211 **DATE:** 01/12/23 SHEET: 1 of 1

		CONDITIONS ENCOUNTEREI	D		_		SAM	IPLE				TESTING AND REMARKS
GROUNDWATER	вц (m) DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
-		Clayey SAND (SC): dark grey; fine to medium; with rootlets.			ND	М						
01/12/23	0.20	Clayey SAND (SC): brown; fine to medium; trace rootlets.				М					-	
Free groundwater observed at 0.35m	-				ND	W					-	
idwater o	0.60	SAND (SP): pale brown; fine to medium.			ND						-	
se grour	ъ ·	Borehole discontinued at 0.70m depth. Limit of investigation.										
-	1 -									- 1 -	-	
_'	. .										•	
NOTES	: ^{(#} Soil ori	gin is "probable" unless otherwise stated. ⁽¹⁾ Consistency/Relative dens	sity shading is	s for visu	ual referenc	e only - n	o correlation b	petweer	n cohes	, ive and (granula	r materials is implied.
PLA	NT: H	and Auger					Douglas					LOGGED: Lambert
		Hand auger to 0.7m DRI @ 0.3m. Coordinates and elevations m typical accuracy of ±0.1m.	neasured	l by d	IGPS w	ith						CASINC: Nil



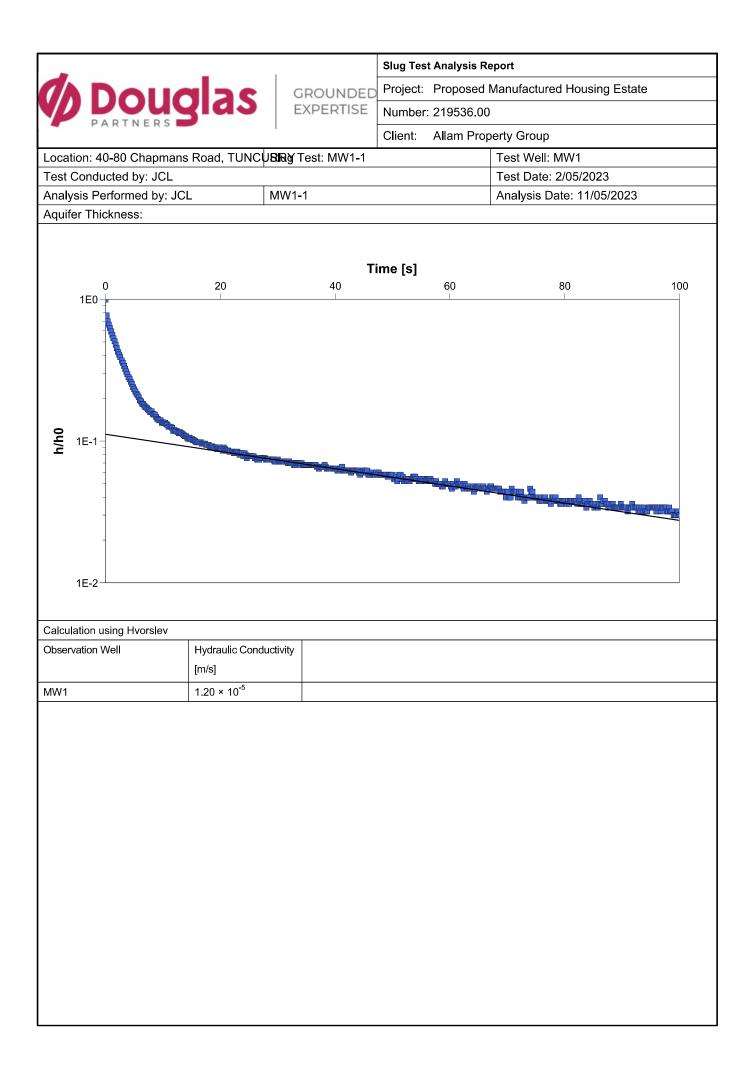
CLIENT: Allam Property Group **PROJECT:** Proposed Manufactured Housing Estate LOCATION: 40-80 Chapmans Road, Tuncurry, NSW

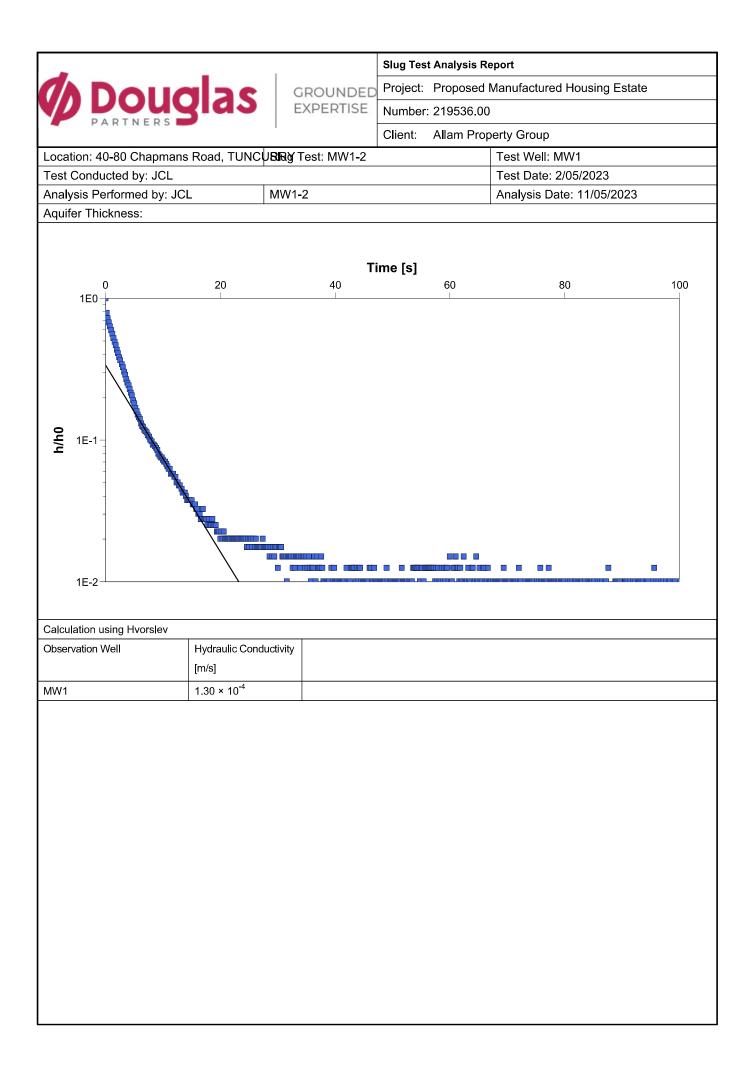
SURFACE LEVEL: 0.8 AHD COORDINATE: E:450943.9, N:6442013.5 PROJECT No: 219536.00 DATUM/GRID: MGA2020 Zone 56 **DIP/AZIMUTH:** 90°/---°

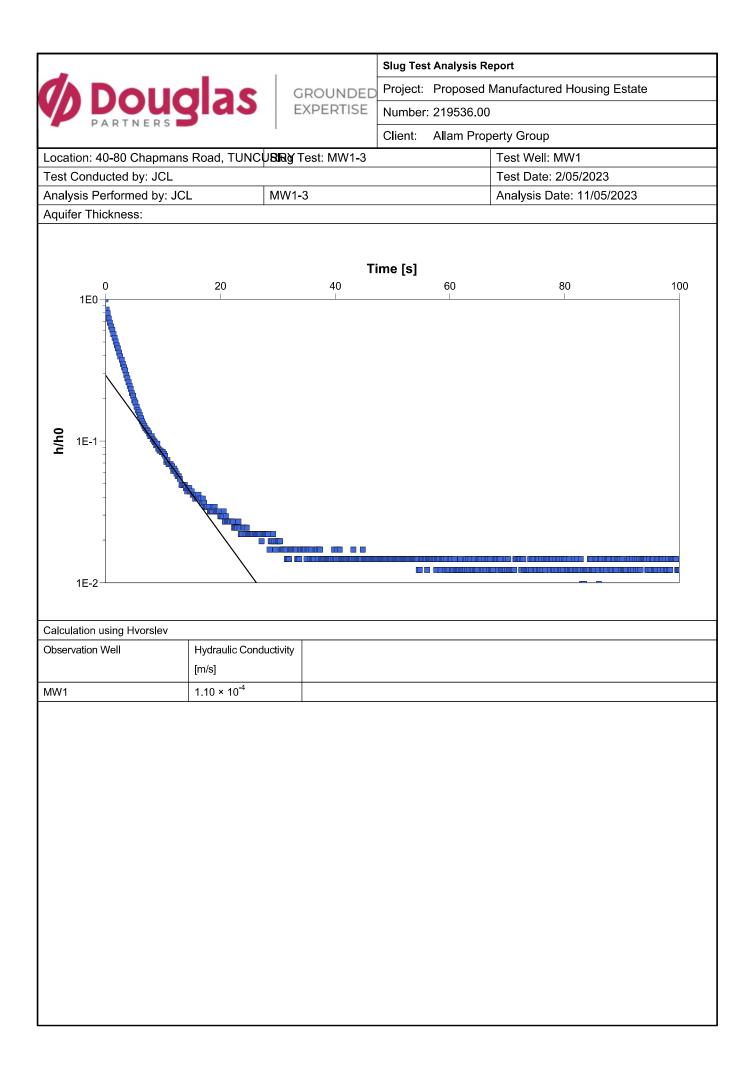
LOCATION ID: 212 **DATE:** 01/12/23 SHEET: 1 of 1

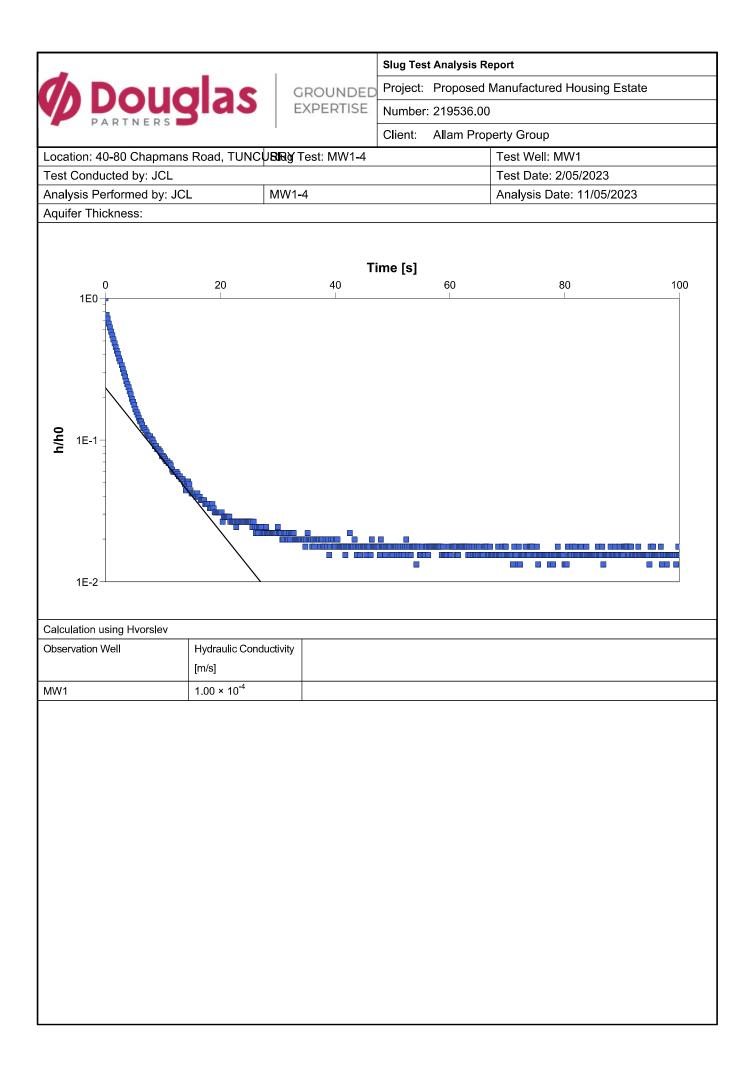
			CONDITIONS ENCOUNTERED					SAN	MPLE				TESTING AND REMARKS
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN ^(#)	CONSIS. ^(*)	MOISTURE	REMARKS	ТҮРЕ	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
-		0.20	Clayey SAND (SC): dark grey; fine to medium; with rootlets.			ND	М				-	-	
▼ EZI		.20	Clayey SAND (SC): black; fine to medium; trace rootlets.			ND	М				-	-	
01/12/23			Borehole discontinued at 0.40m depth.				W						
	٩ 	-											
NOTES	5: 件5	Soil ori	gin is "probable" unless otherwise stated. "Consistency/Relative densiț	y shading i	s for visu	al referenc	e only - n	o correlation	betweer	n cohes	ive and	 granula	r materials is implied.
			and Auger		(OPERA	TOR:	Douglas	s Part	ners			LOGGED: Lambert
			Hand auger to 0.4m DRI @ 0.3m. Coordinates and elevations me typical accuracy of ±0.1m.	easurec	l by d	GPS w	ith						CASING: Nil

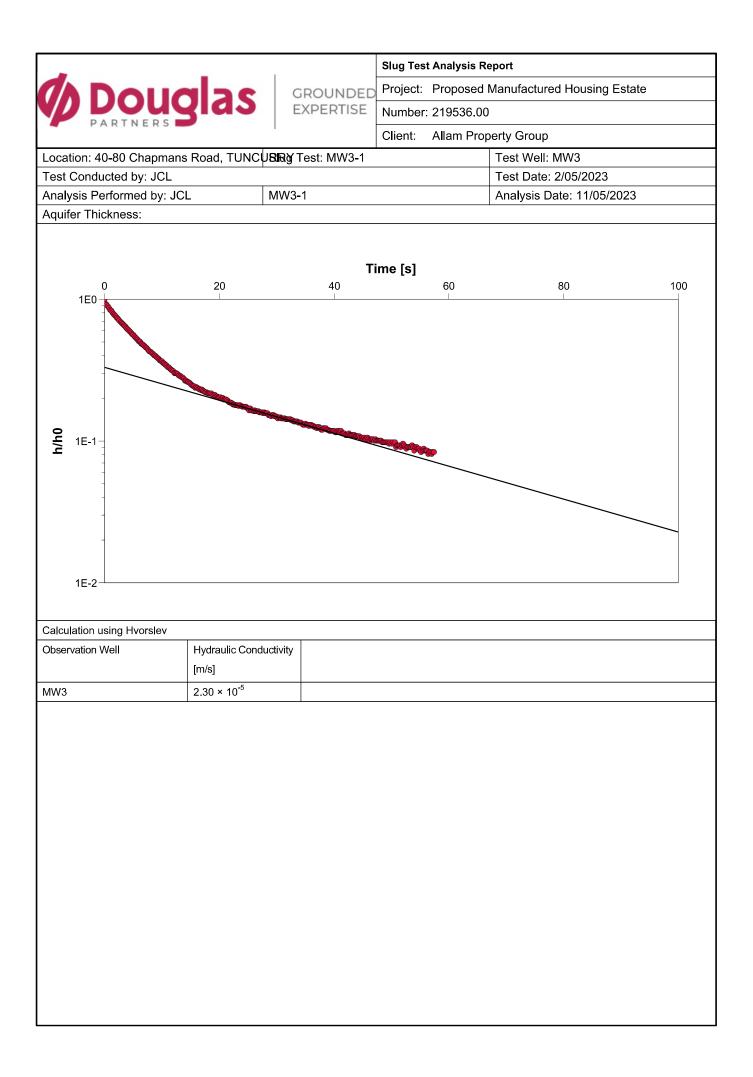


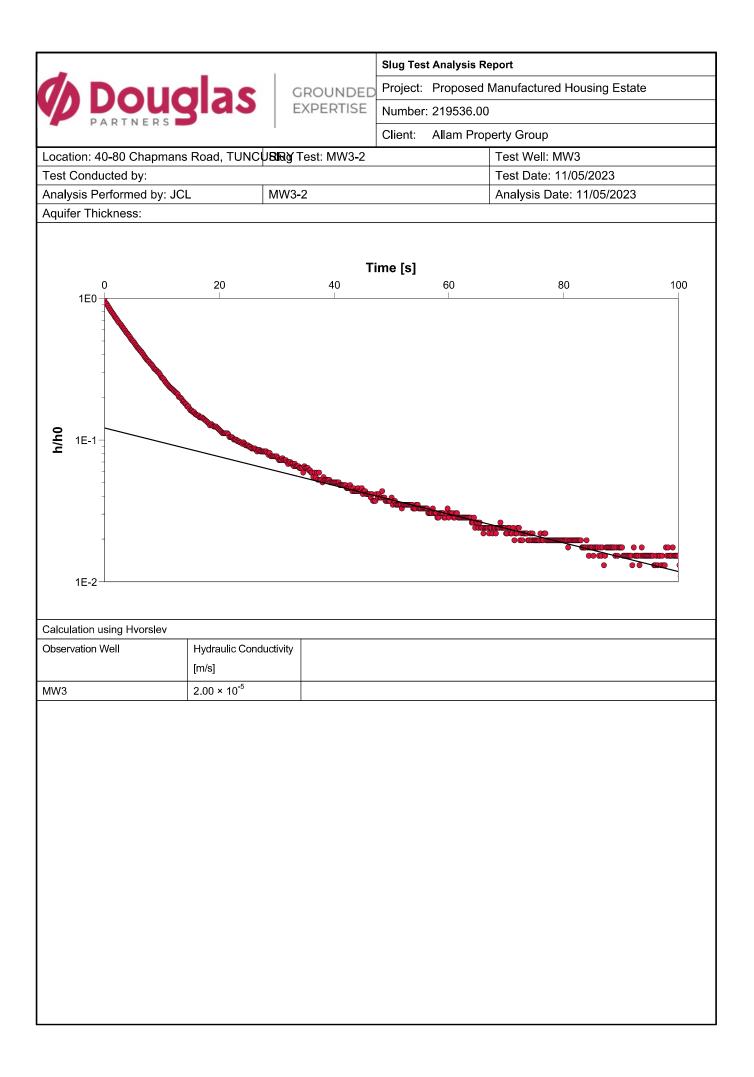


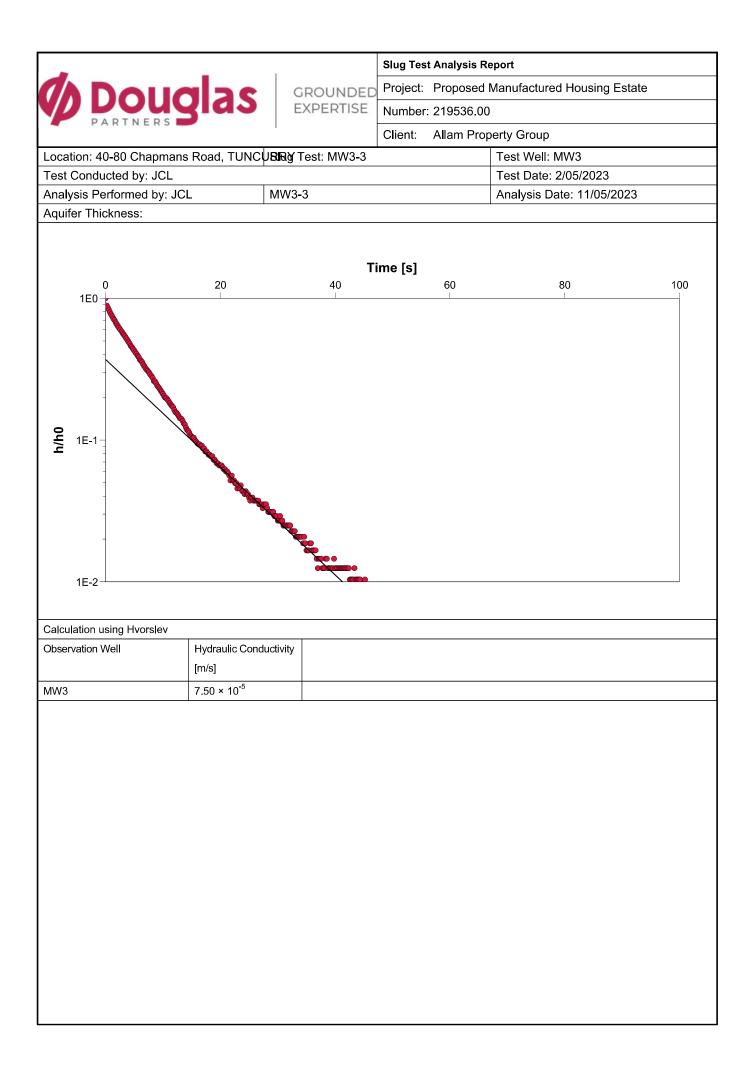


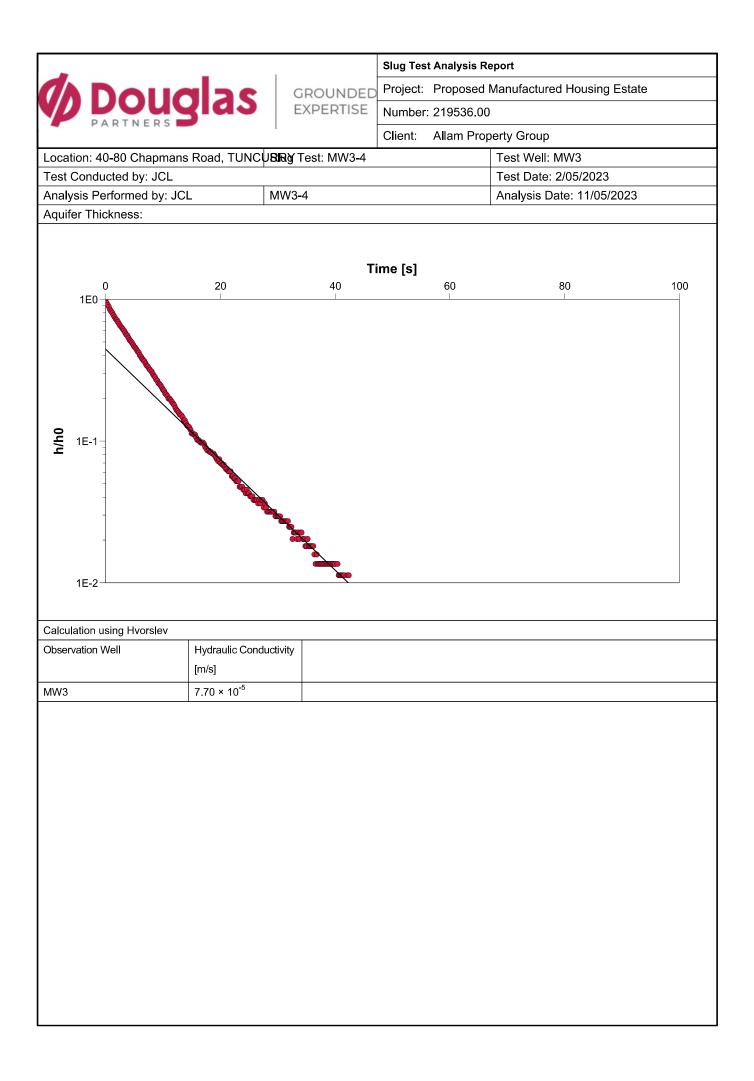


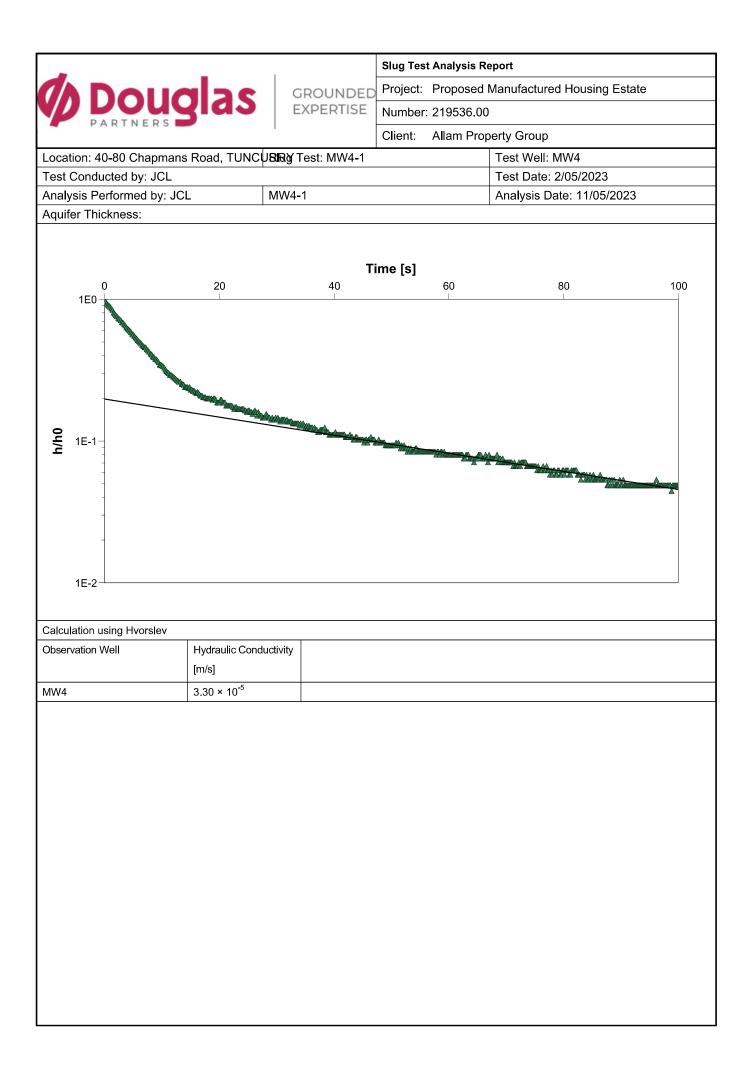


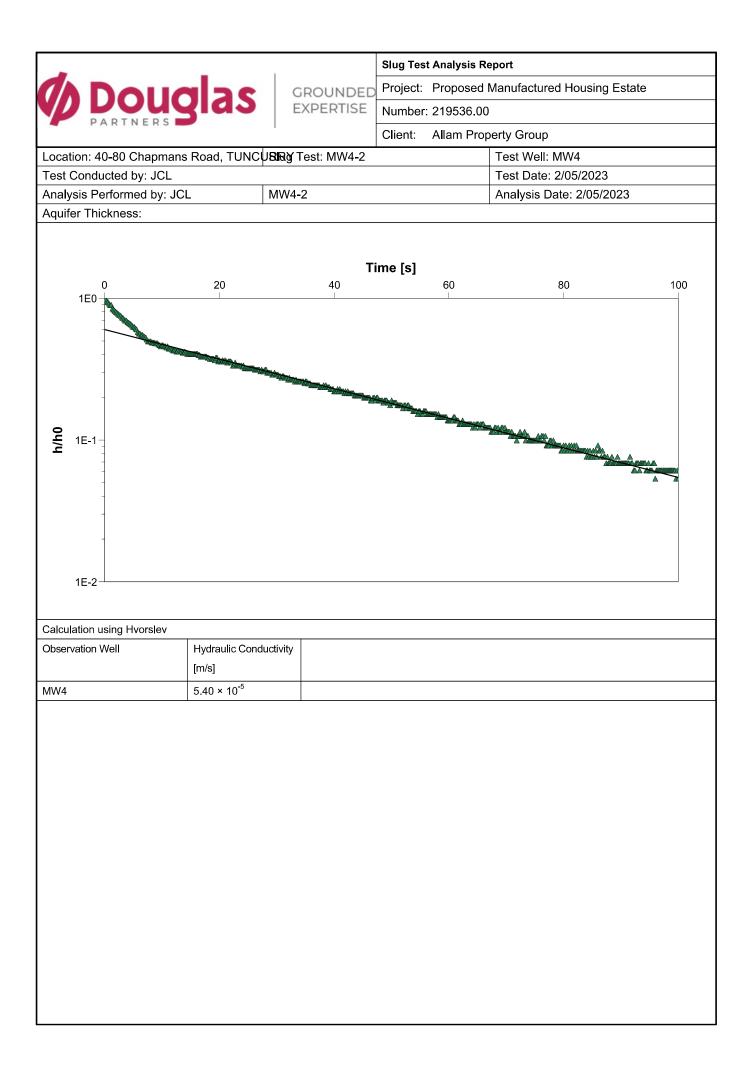


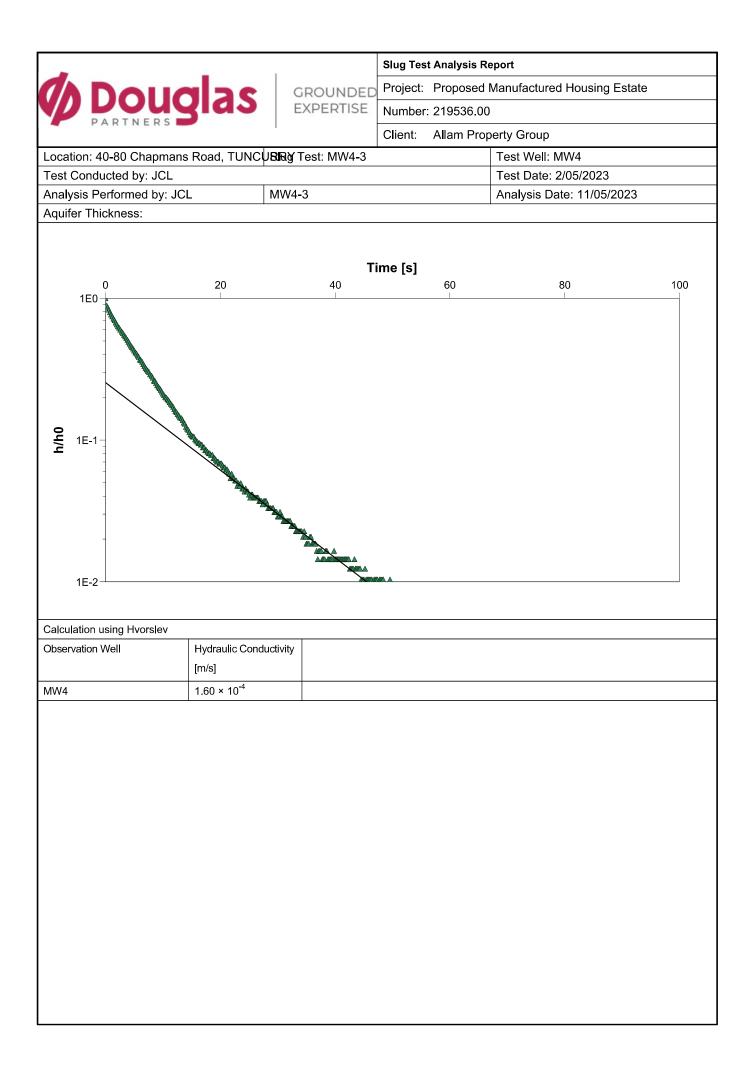












			Slug Test	Analysis R	eport	
		GROUNDEI	Project:	Proposed	Manufactured Housing Est	ate
	zias	EXPERTISE		219536.00		
PARTNERS					perty Group	
Location: 40-80 Chapmans		NTost MW4-4			Test Well: MW4	
Test Conducted by: JCL		g rest. 1004-4			Test Date: 2/05/2023	
Analysis Performed by: JC		V4 - 4			Analysis Date: 11/05/202	3
Aquifer Thickness:		<u>, , , , , , , , , , , , , , , , , , , </u>			7 (nalysis Bate: 11/06/202	0
		-	Гime [s]			
0 1E0-	20	40	[0]	60	80	100
OYJY 1E-1						
1E-2 ^{_⊥} Calculation using Hvorslev						
	Hydraulic Conductivi	ty				
Calculation using Hvorslev	Hydraulic Conductivi [m/s] 1.80 × 10 ⁻⁴	ty				

